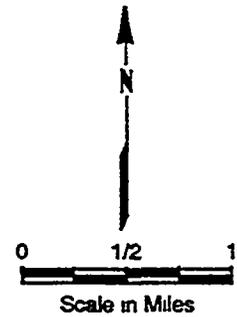


Explanation

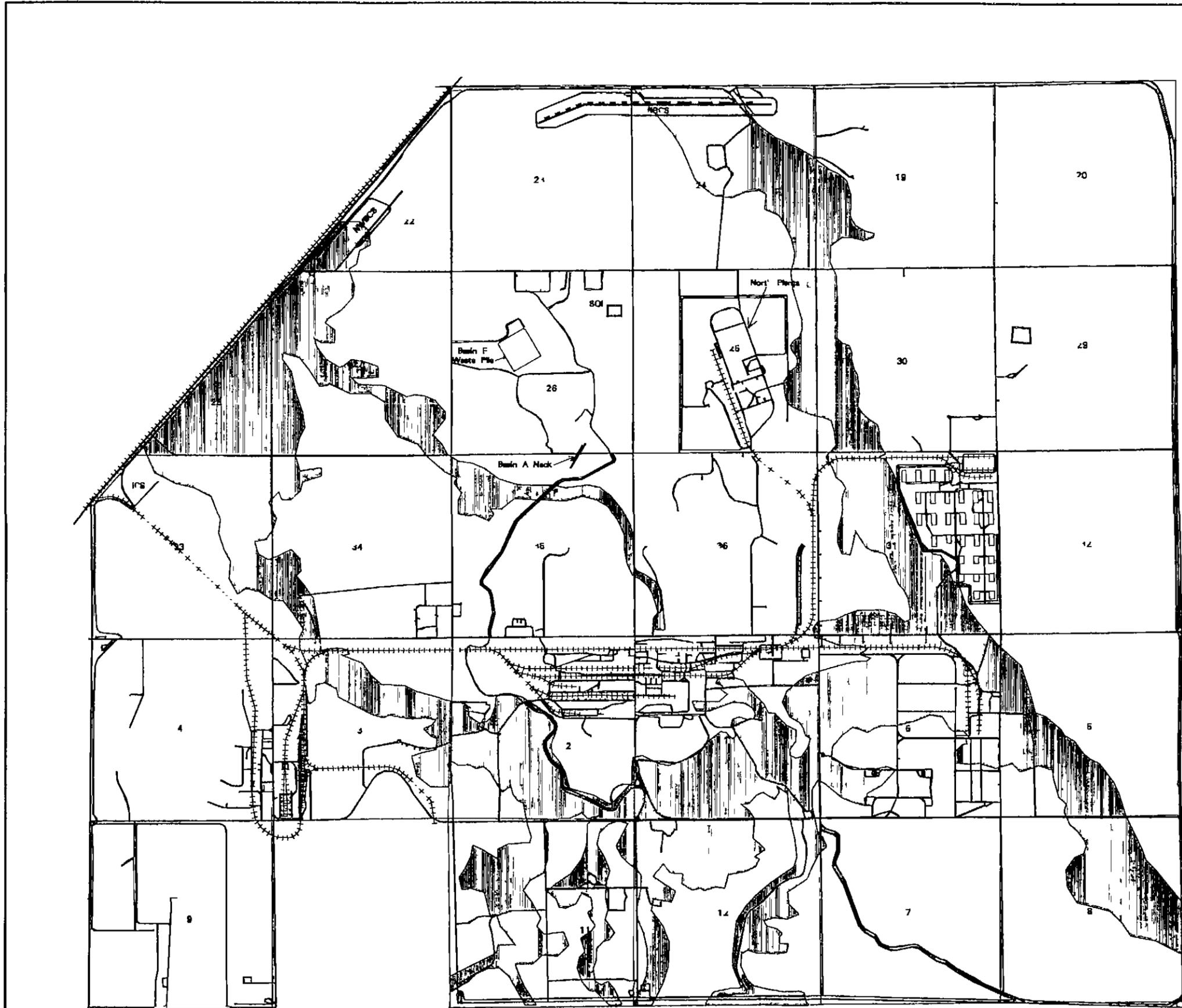
- 2 Section Numbers
- Section Lines
- Waterways Experiment Station (WES) 1983
- U S Army Toxic and Hazardous Materials Agency (USATHAMA), 1984
- Ebasco Services Incorporated (EBASCO), 1988



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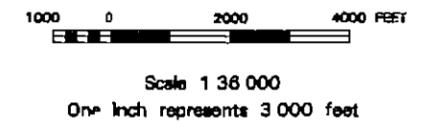
Prepared by:
 Harding Lawson Associates

Figure 4 1
 Proposed Landfill Locations from
 Previous Landfill Siting Studies



Explanation

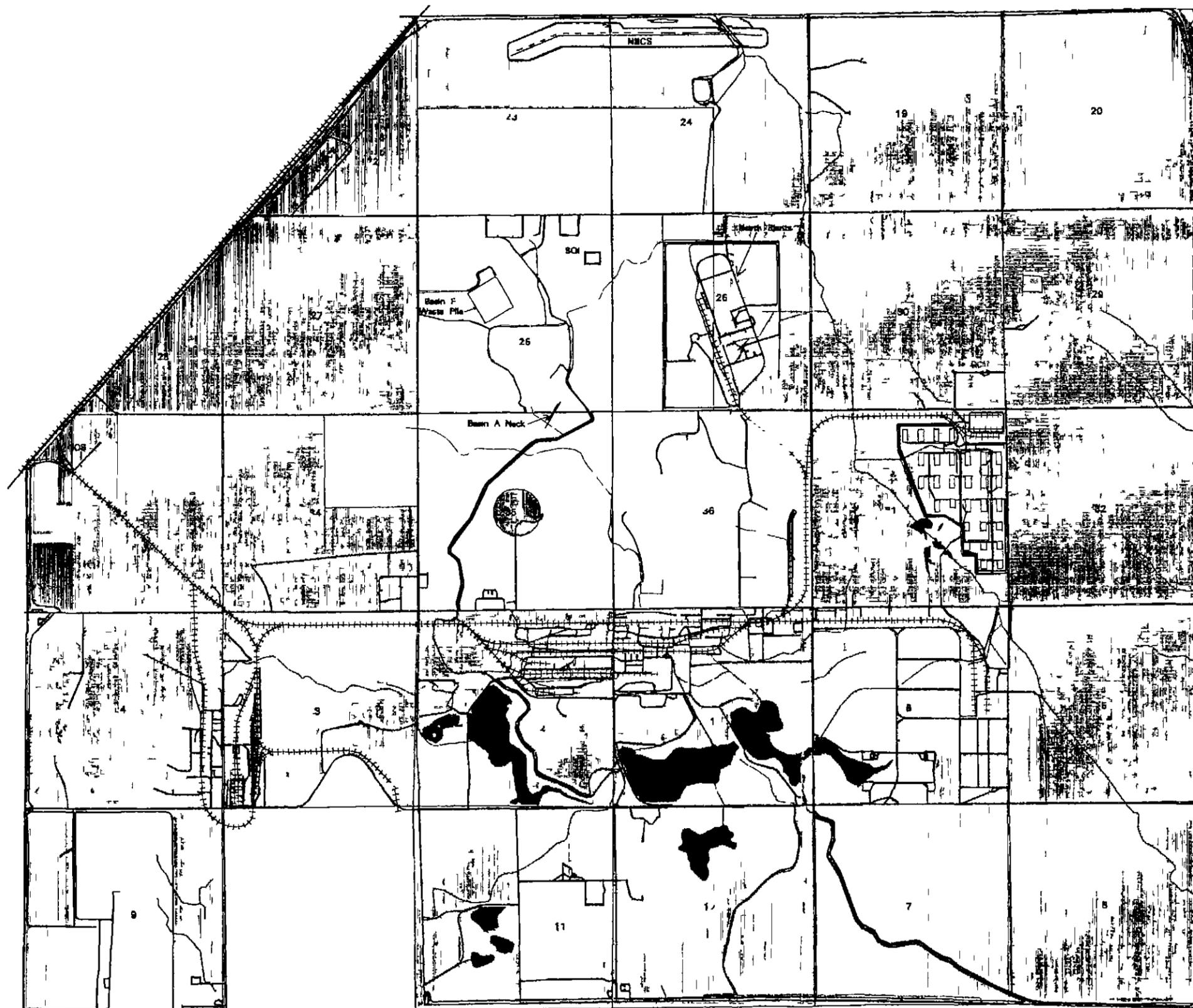
-  100-Year Floodplan
-  Road
-  Railroad
-  Section line
- 1-34 Section number
- NBCS North Boundary Containment System
- MWBCS Northwest Boundary Containment System
- ICS Irondale Containment System
- SQI Submerged Quench Incinerator



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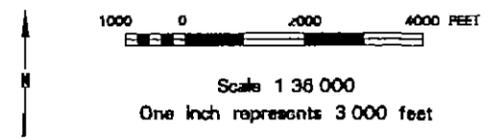
Figure 4.2
 100-Year Floodplain Map

(U.S. Army Corps of Engineers, 1983)



Explanation

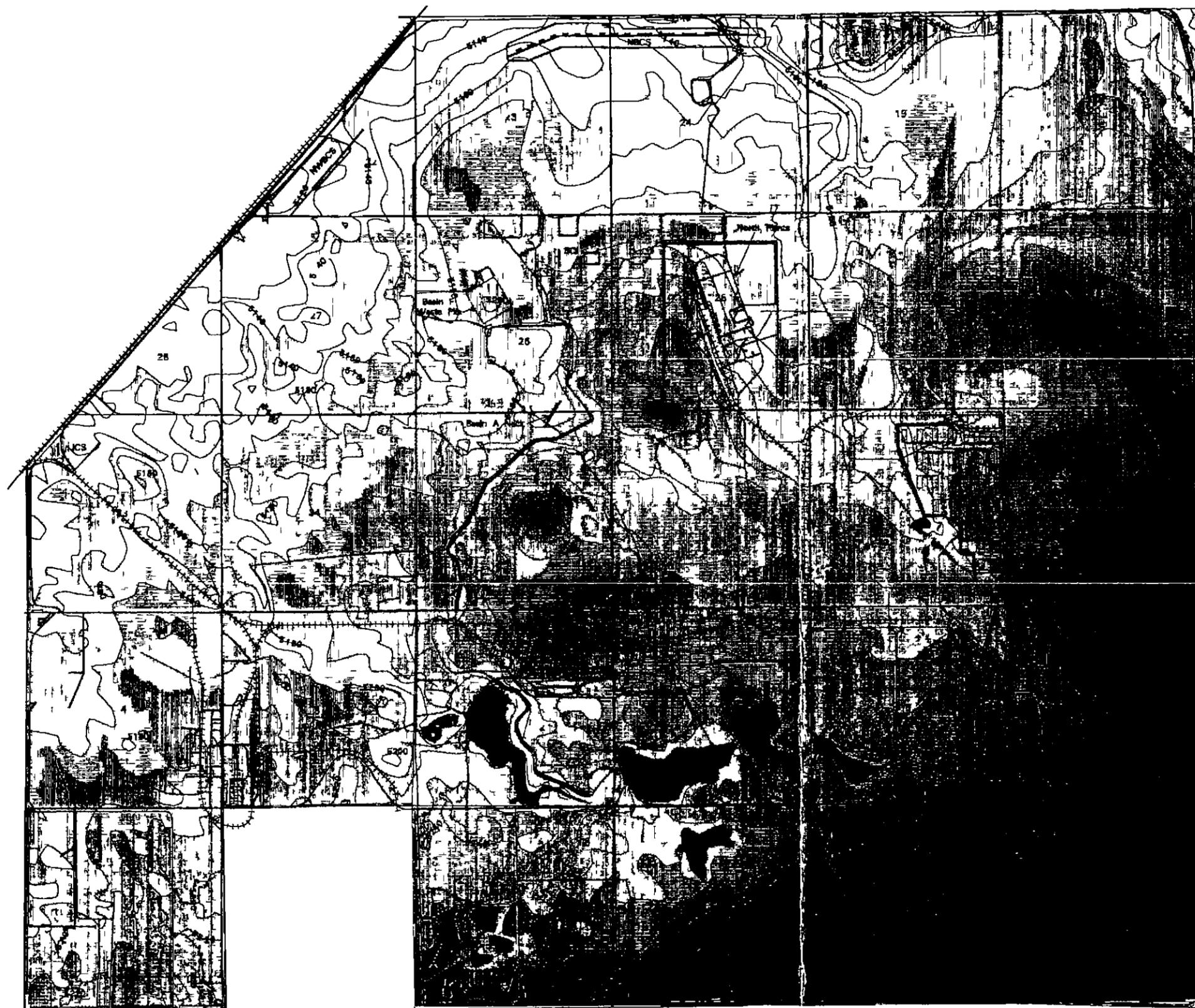
-  United States Fish and Wildlife Service (USFWS) approved landfill sites
-  Surface Water
-  Stream
-  Road
-  Railroad
-  Section line
- 1-2 Section number
- NBCS North Boundary Containment System
- NWBCS Northwest Boundary Containment System
- ICS Irondale Containment System
- SQI Submerged Quench Incinerator



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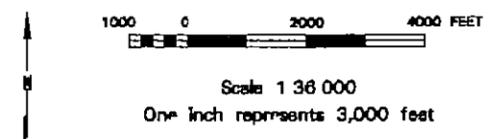
Prepared by
 Harding Lawson Associates

Figure 4.3
 United States Fish and Wildlife Service
 Approved Hazardous Waste Landfill Sites
 (USFWS, 1994)



Explanation

- 5110 feet
- 5340 feet
- 10-foot contour interval
- Surface Water
- Stream
- Road
- Railroad
- Section line
- 12 Section number
- NBCS North Boundary Containment System
- NWBCS Northwest Boundary Containment System
- ICS Irondale Containment System
- SQI Submerged Quench Incinerator
- PMRMA Program Manager Rocky Mountain Arsenal



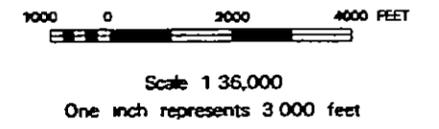
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Figure 4.4
 Onpost Topographic Elevation
 Contour Map
 (PMRMA, 1988)

Explanation

-  Groundwater Contamination Plume
-  Road
-  Railroad
-  Section line
-  Section number
-  NBCS North Boundary Containment System
-  NWBCS Northwest Boundary Containment System
-  ICS Irondale Containment System
-  SQI Submerged Quench Incinerator

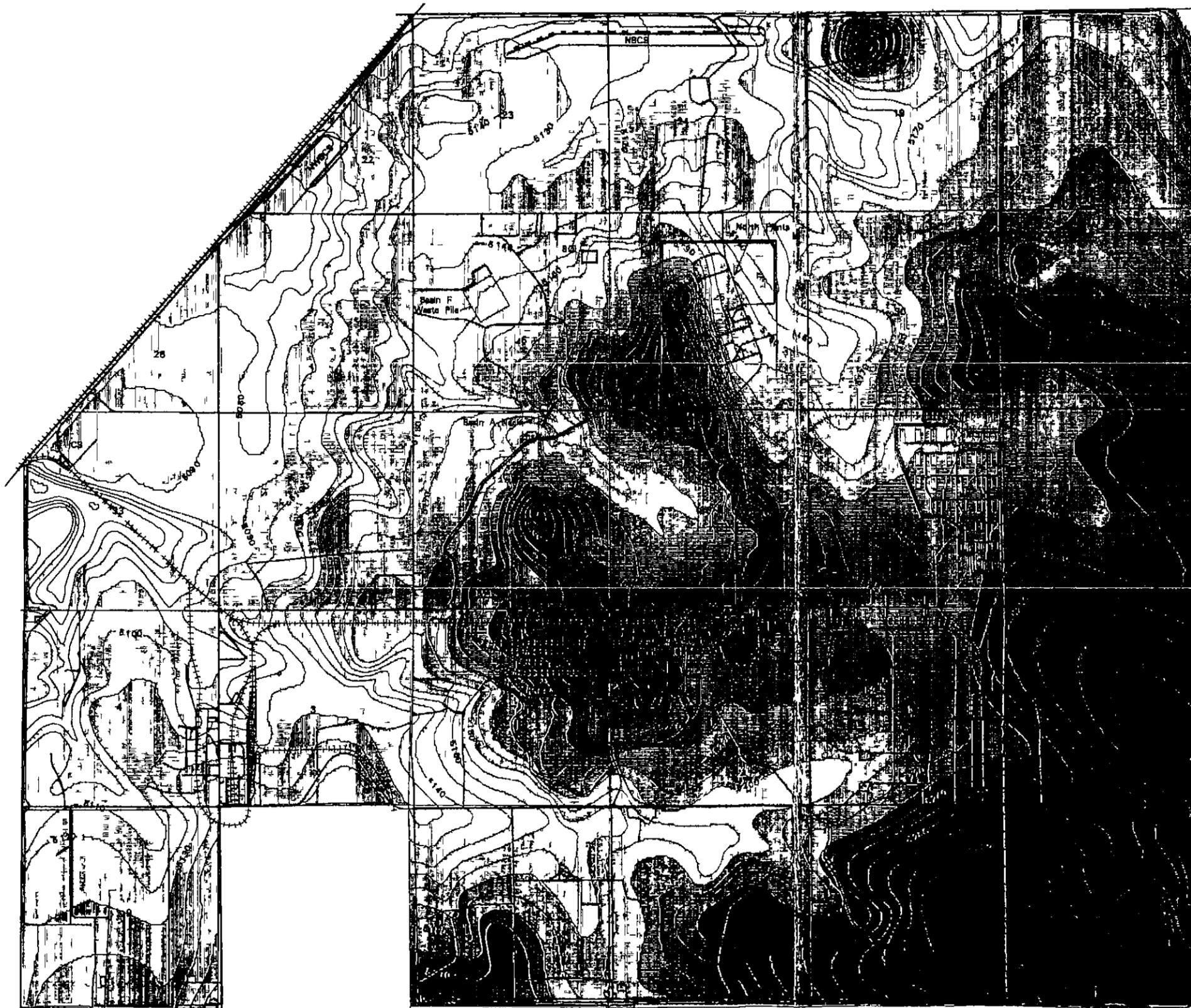


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Figure 4.5
Groundwater Contamination
Plume Map of RMA
(EBASCO, 1994)





Explanation

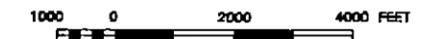
5040 feet



5310 feet

- 10-foot contour interval
- == Road
- +++++ Railroad
- Section line

- 1,2 Section number
- NBCS North Boundary Containment System
- NWBCS Northwest Boundary Containment System
- ICS Irondale Containment System
- SQI Submerged Quench Incinerator
- HLA Harding Lawson Associates



Scale 1:36 000

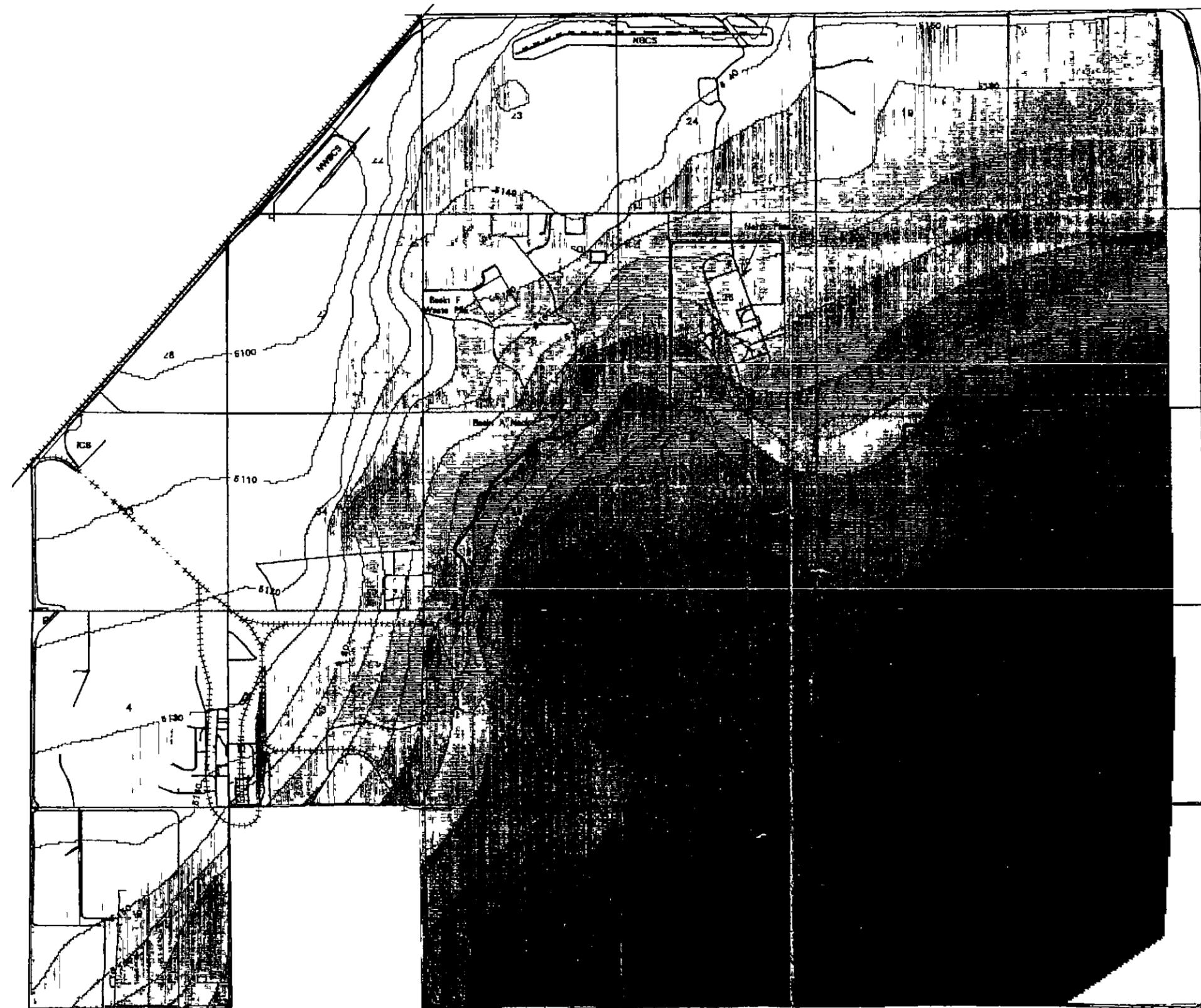
One inch represents 3 000 feet

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Figure 4.7

Onpost Bedrock
Elevation Contour Map
(HLA, 1992)



Explanation

5090 feet



5300 feet

— 10-foot contour interval

== Road

++++ Railroad

— Section line

12 Section number

NBCS North Boundary Containment System

NWBCS Northwest Boundary Containment System

ICS Irondale Containment System

SQI Submerged Quench Incinerator

HLA Harding Lawson Associates
 1000 0 2000 4000 FEET

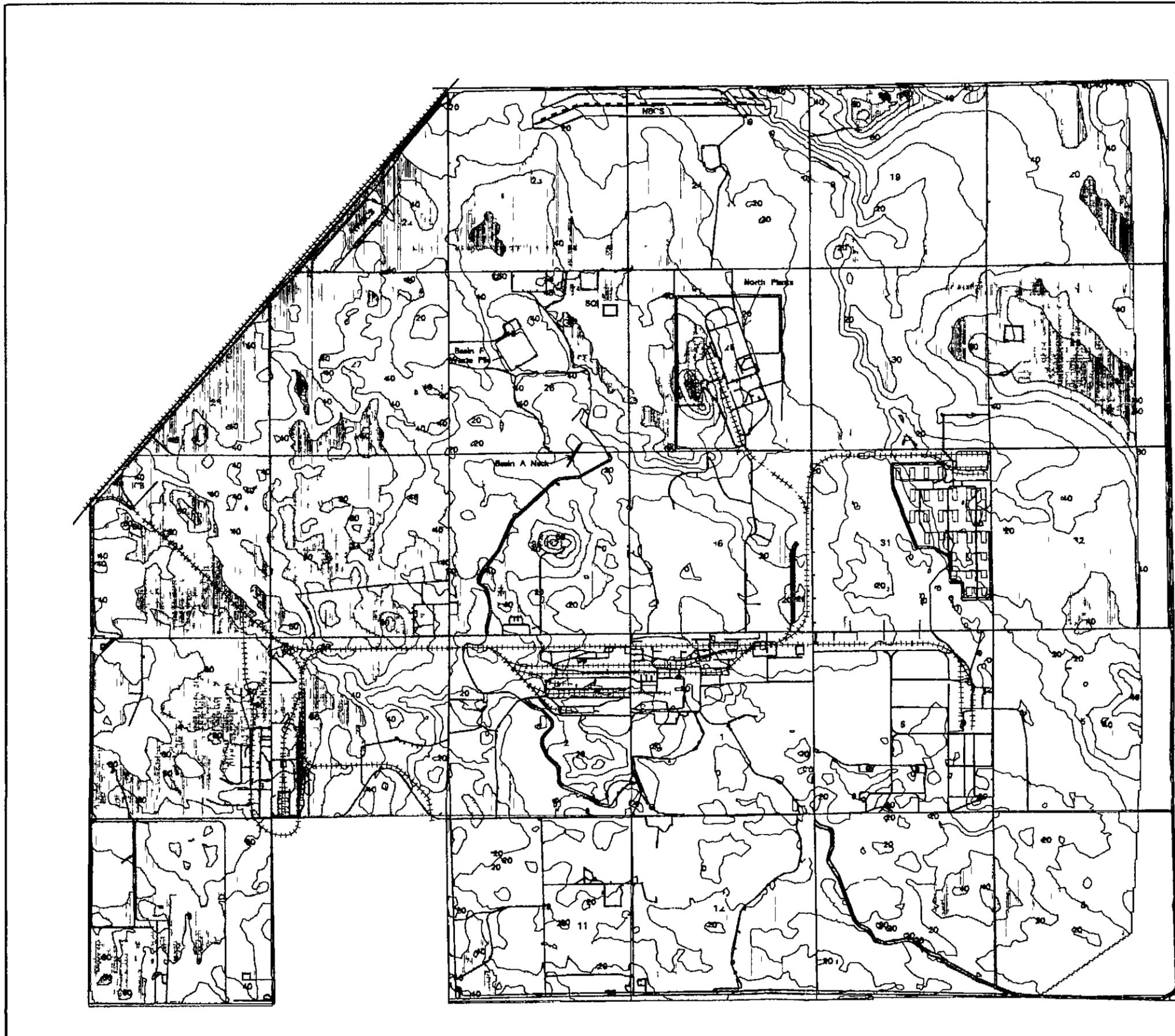
Scale 1:36,000

One inch represents 3,000 feet

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Figure 4.8

Onpost Winter 1991 Water-level Elevation Contour Map
 (HLA, 1992)



Explanation

0 feet



90 feet

10-foot contour interval

Road

Railroad

Section line

12 Section number

NBCS North Boundary Containment System

NWBCS Northwest Boundary Containment System

ICS Irondale Containment System

SQI Submerged Quench Incinerator

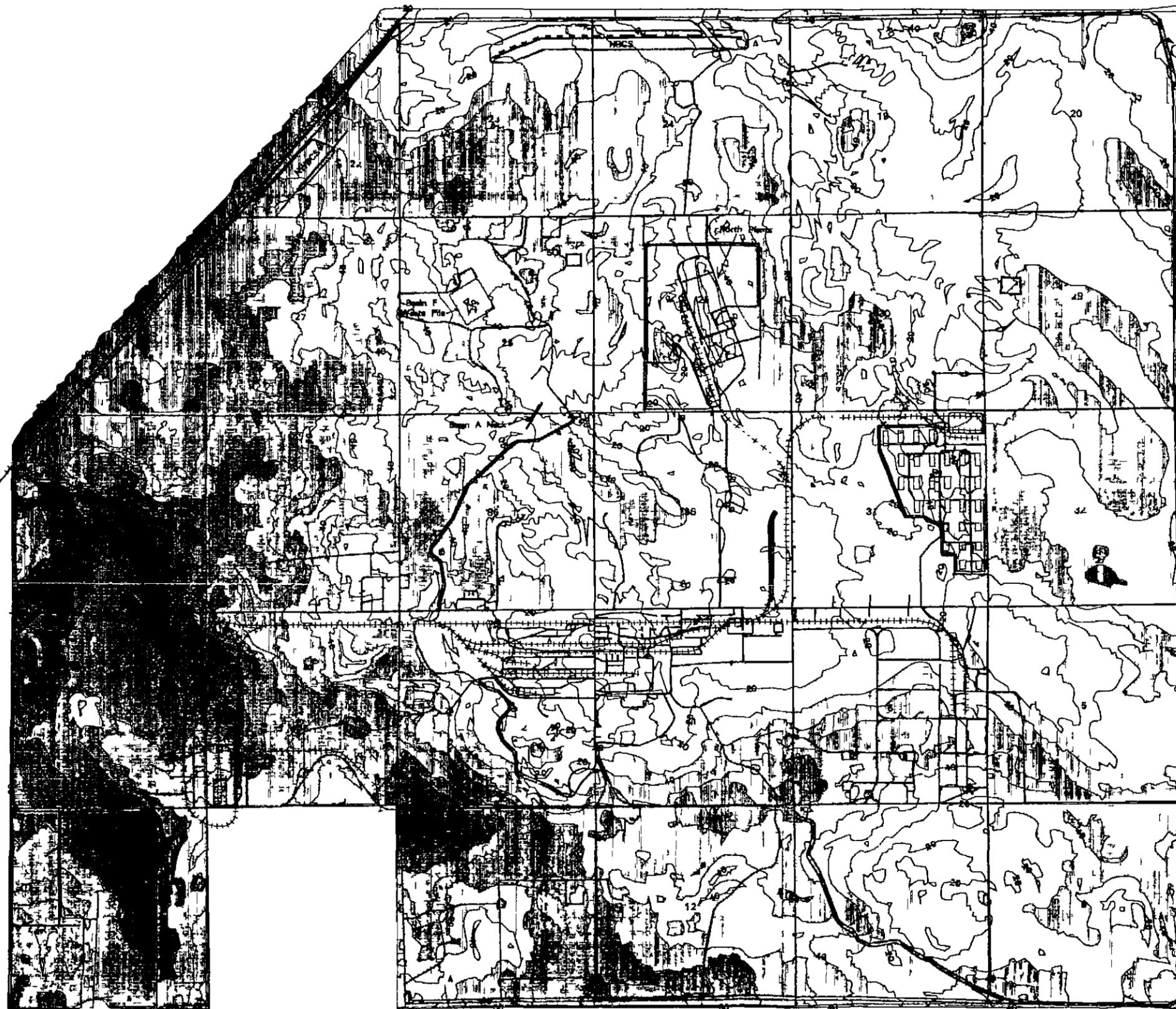


Scale 1:36 000

One inch represents 3 000 feet

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Figure 4.9
 Onpost Approximate Depth
 to Groundwater Contour Map



Explanation

0 feet



140 feet

10-foot contour interval

Road

Railroad

Section line

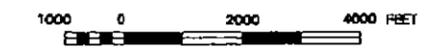
12 Section number

NBCS North Boundary Containment System

NWBCS Northwest Boundary Containment System

ICS Irondale Containment System

SQI Submerged Quench Incinerator



Scale 1:38,000

One inch represents 3,000 feet

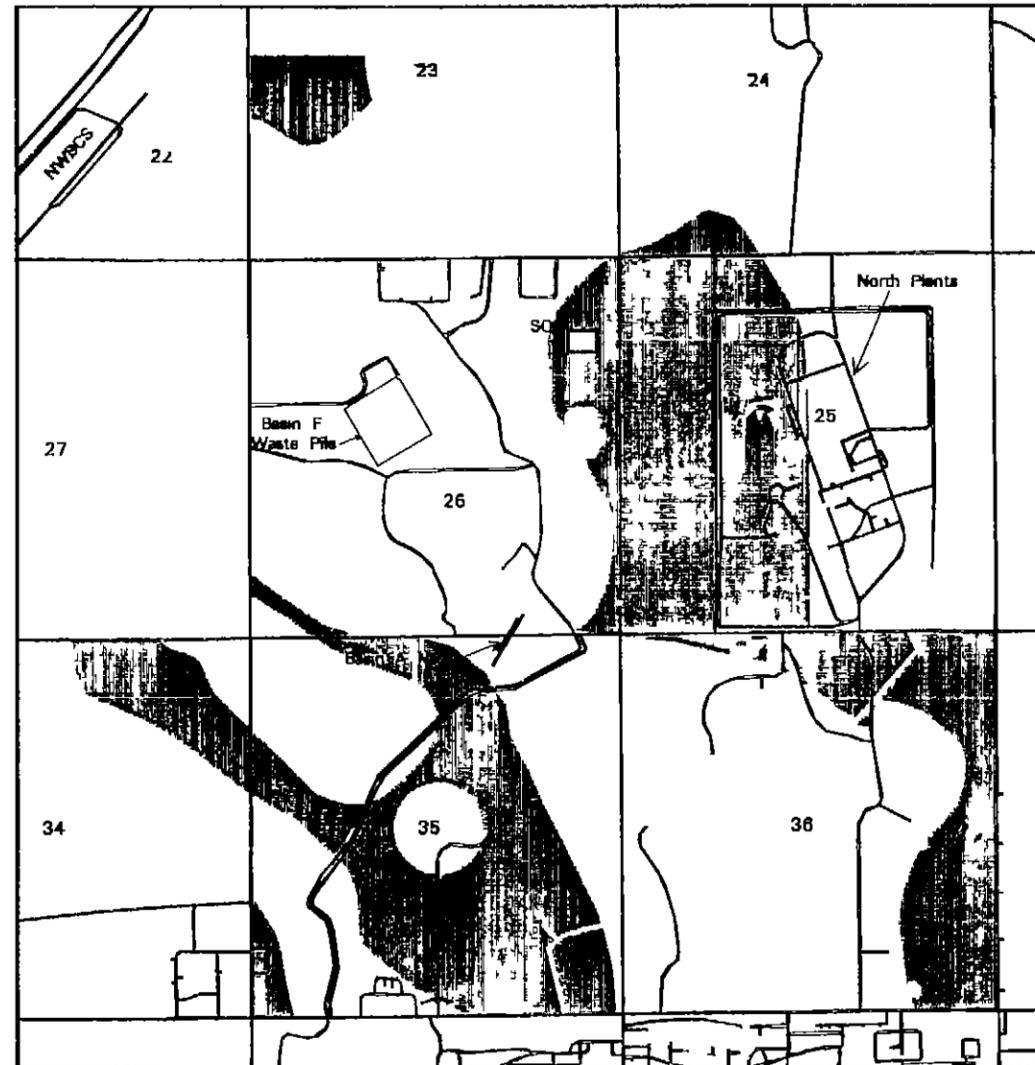
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Figure 4.10

Onpost Approximate Depth
 to Bedrock Contour Map

Potential Landfill Areas

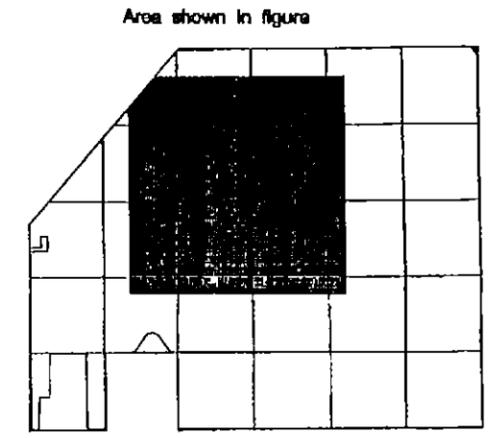


Secondary Landfill Siting Criteria

The values listed below were used to determine siting potential based on depth to groundwater, depth to bedrock, and depth to saturated alluvium.

Maximize	
Depth to groundwater (in feet)	
10 to 70 feet	Not suitable
70 to 90 feet	Suitable
Minimize	
Depth to bedrock (in feet)	
0 to 10 feet	Suitable
10 to 80 feet	Not Suitable

Note: Secondary landfill siting criteria includes unsaturated alluvium.

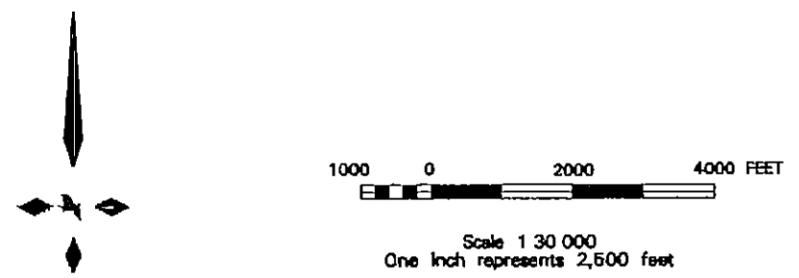


Rocky Mountain Arsenal

Note
The "Suitable Areas" identified in this figure are the result of a spatial analysis performed to eliminate those areas at RMA deemed "unsuitable" with respect to the primary and secondary landfill siting criteria. The primary landfill siting criteria include avoidance of wetlands sensitive habitats, 100-year floodplain organic groundwater contamination plumes, and human health exceedance areas as listed in Table 4.2. The secondary landfill siting criteria are identified in the box titled secondary landfill siting criteria.

- Suitable areas based on primary landfill siting criteria
- Suitable areas based on secondary landfill siting criteria
- Road
- Section number
- Northwest Boundary Containment System
- Submerged Quench Incinerator

Total suitable acres based on primary and secondary landfill siting criteria 1
Maximum contiguous suitable acres 1

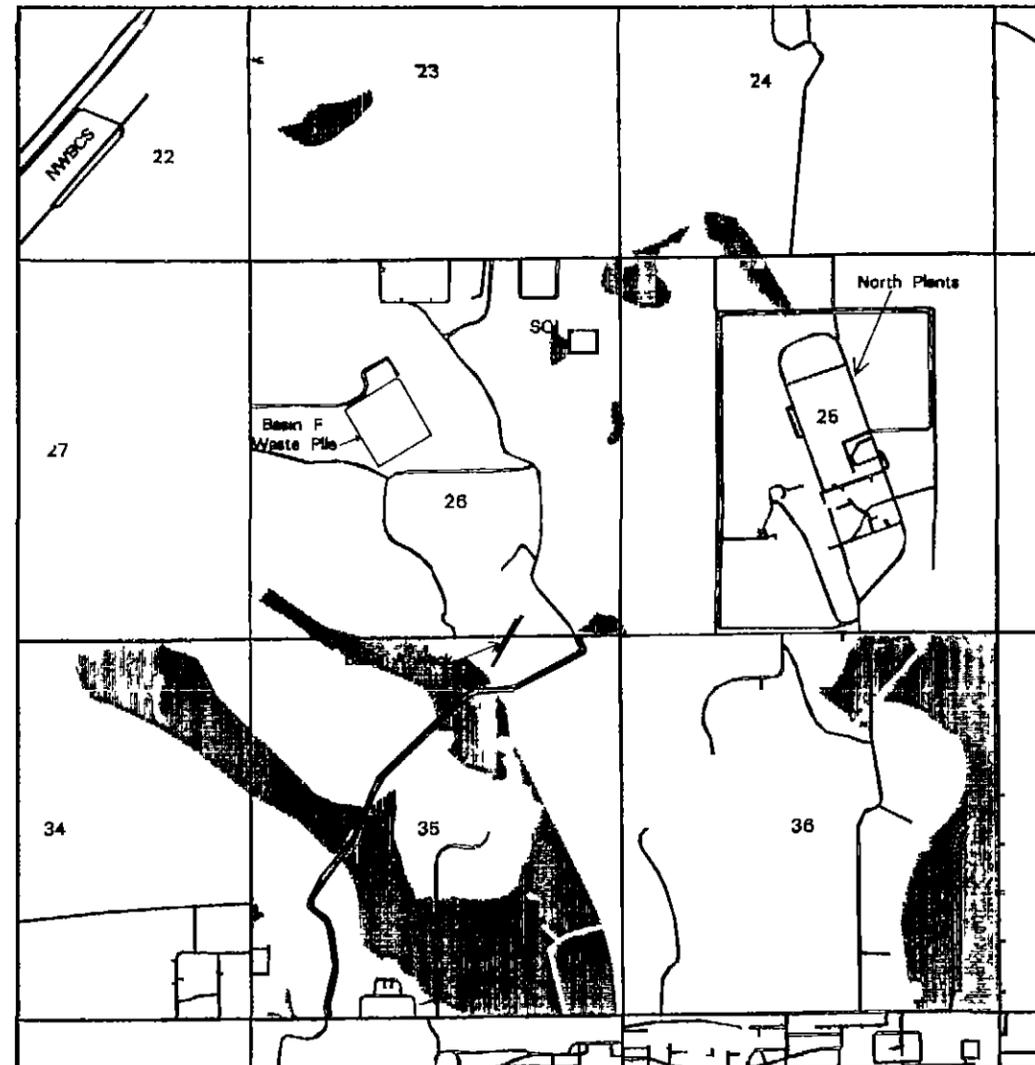


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Figure 4-11 - Applied Selection Criteria - Depth to Groundwater Greater Than or Equal to 70 feet, and Depth to Bedrock Less Than or Equal to 10 feet

Potential Landfill Areas



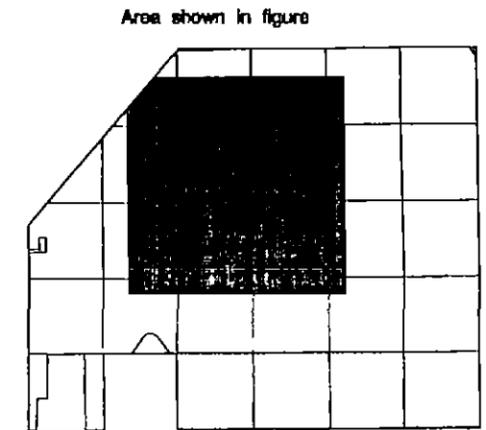
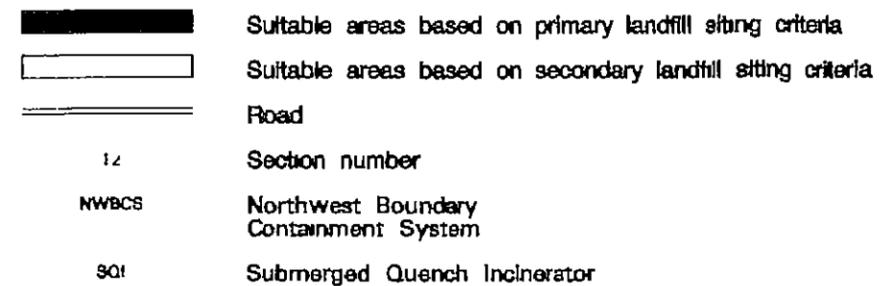
Secondary Landfill Siting Criteria

The values listed below were used to determine siting potential based on depth to groundwater depth to bedrock and depth to saturated alluvium

Maximize
Depth to groundwater (in feet)
10 to 40 feet Not suitable
40 to 90 feet Suitable

Minimize
Depth to bedrock (in feet)
0 to 40 feet Suitable
40 to 80 feet Not Suitable

Note Secondary landfill siting criteria includes unsaturated alluvium



Rocky Mountain Arsenal

Note
The "Suitable Areas" identified in this figure are the result of a spatial analysis performed to eliminate those areas at RMA deemed "unsuitable" with respect to the primary and secondary landfill siting criteria. The primary landfill siting criteria include avoidance of wetlands sensitive habitats 100-year floodplain organic groundwater contamination plumes, and human health exceedance areas as listed in Table 4.2. The secondary landfill siting criteria are identified in the box titled secondary landfill siting criteria.

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Total suitable acres based on primary and secondary landfill siting criteria. 469
Maximum contiguous suitable acres 371

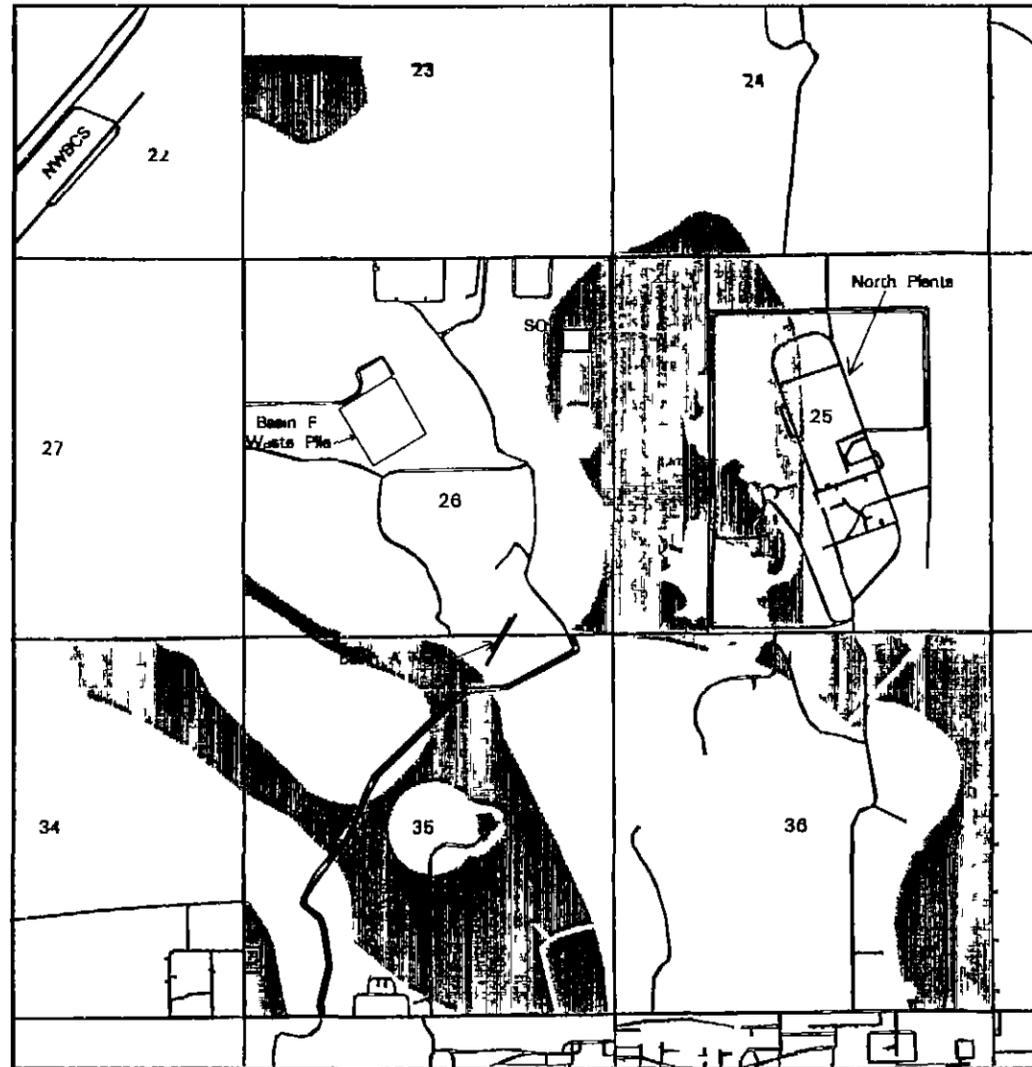


1000 0 2000 4000 FEET

Scale 1:30,000
One inch represents 2,500 feet

Figure 4.12 - Applied Selection Criteria - Depth to Groundwater Greater Than or Equal to 40 feet, and Depth to Bedrock Less Than or Equal to 40 feet

Potential Landfill Areas



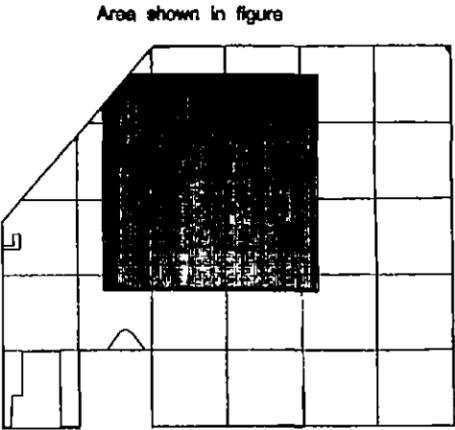
Secondary Landfill Siting Criteria

The values listed below were used to determine siting potential based on depth to groundwater, depth to bedrock, and depth to saturated alluvium.

Maximize
 Depth to groundwater (in feet)
 10 to 60 feet Not suitable
 60 to 90 feet Suitable

Minimize
 Depth to bedrock (in feet)
 0 to 20 feet Suitable
 20 to 80 feet Not Suitable

Note: Secondary landfill siting criteria includes unsaturated alluvium.

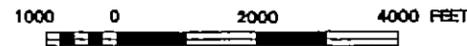


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Note
 The "Suitable Areas" identified in this figure are the result of a spatial analysis performed to eliminate those areas at RMA deemed "unsuitable" with respect to the primary and secondary landfill siting criteria. The primary landfill siting criteria include avoidance of wetlands, sensitive habitats, 100-year floodplain, organic groundwater contamination plumes, and human health exceedance areas as listed in Table 4.2. The secondary landfill siting criteria are identified in the box titled secondary landfill siting criteria.

-  Suitable areas based on primary landfill siting criteria
-  Suitable areas based on secondary landfill siting criteria
-  Road
-  Section number
-  Northwest Boundary Containment System
-  Submerged Quench Incinerator

Total suitable acres based on primary and secondary landfill siting criteria 81
 Maximum contiguous suitable acres 76



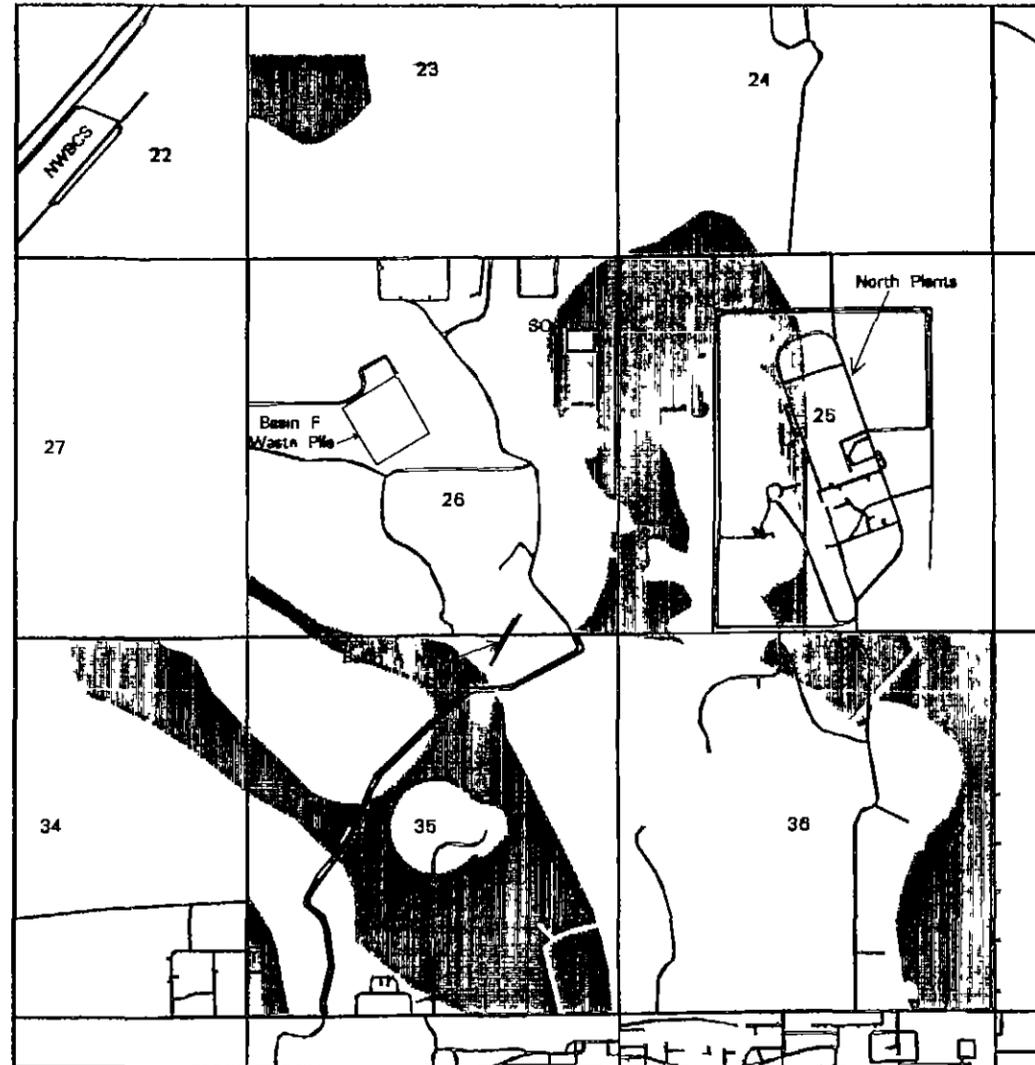
Scale 1:30,000
 One inch represents 2,500 feet

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Figure 4.13 - Applied Selection Criteria - Depth to Groundwater Greater Than or Equal to 60 feet, and Depth to Bedrock Less Than or Equal to 20 feet

Potential Landfill Areas



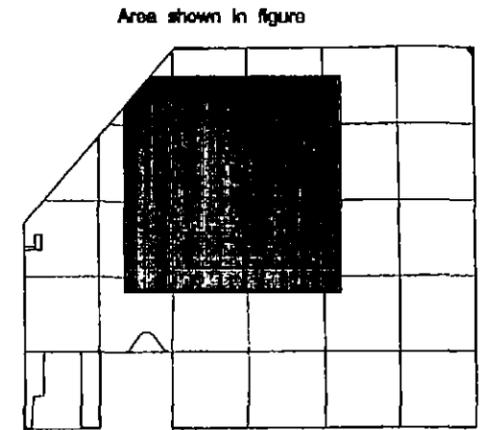
Secondary Landfill Siting Criteria

The values listed below were used to determine siting potential based on depth to groundwater depth to bedrock and depth to saturated alluvium

Maximize	
Depth to groundwater (In feet)	
10 to 60 feet	Not suitable
60 to 90 feet	Suitable
Minimize	
Depth to bedrock (In feet)	
0 to 40 feet	Suitable
40 to 80 feet	Not Suitable

Note Secondary landfill siting criteria includes unsaturated alluvium

- Suitable areas based on primary landfill siting criteria
- Suitable areas based on secondary landfill siting criteria
- Road
- 12 Section number
- NWBCS Northwest Boundary Containment System
- SQI Submerged Quench Incinerator



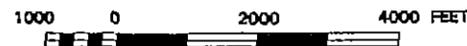
Rocky Mountain Arsenal

Note
The "Suitable Areas" identified in this figure are the result of a spatial analysis performed to eliminate those areas at RMA deemed "unsuitable" with respect to the primary and secondary landfill siting criteria. The primary landfill siting criteria include avoidance of wetlands sensitive habitats 100-year floodplain organic groundwater contamination plumes, and human health exceedance areas as listed in Table 4.2. The secondary landfill siting criteria are identified in the box titled secondary landfill siting criteria.

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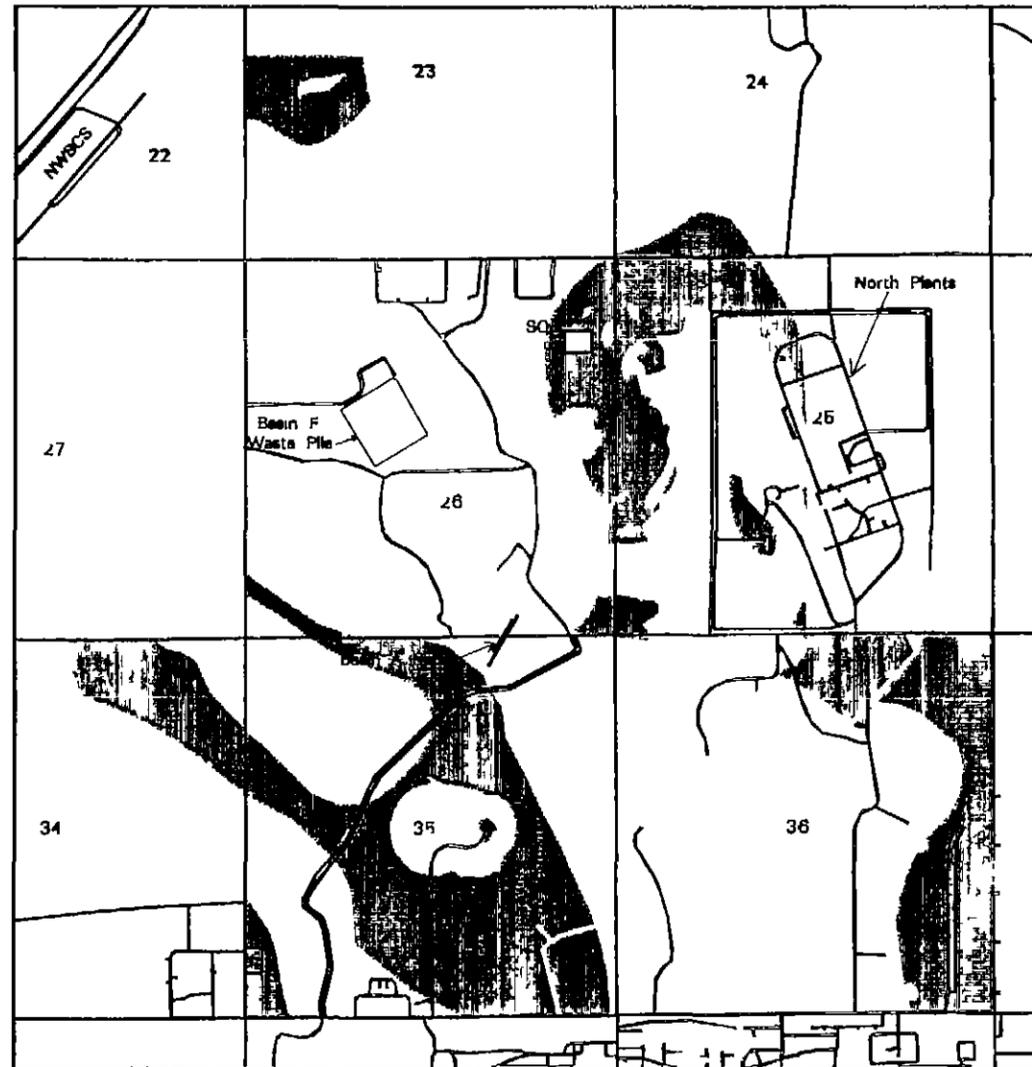
Total suitable acres based on primary and secondary landfill siting criteria. 147
Maximum contiguous suitable acres 138



Scale 1:30,000
One inch represents 2,500 feet

Figure 4.14 - Applied Selection Criteria - Depth to Groundwater Greater Than or Equal to 60 feet, and Depth to Bedrock Less Than or Equal to 40 feet

Potential Landfill Areas

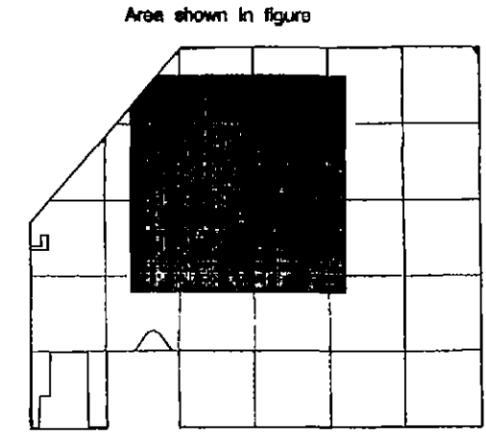


Secondary Landfill Siting Criteria

The values listed below were used to determine siting potential based on depth to groundwater, depth to bedrock, and depth to saturated alluvium.

Maximize	
Depth to groundwater (in feet)	
10 to 50 feet	Not suitable
50 to 90 feet	Suitable
Minimize	
Depth to bedrock (in feet)	
0 to 30 feet	Suitable
30 to 80 feet	Not Suitable

Note: Secondary landfill siting criteria includes unsaturated alluvium.

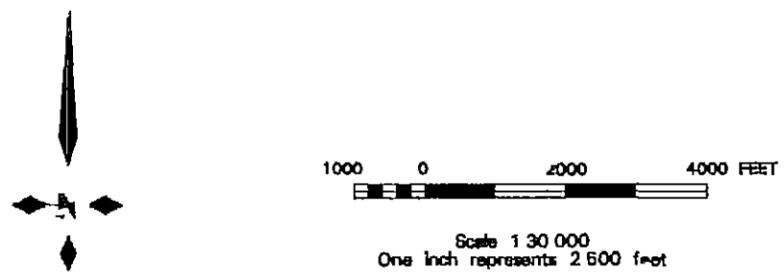


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Note
The "Suitable Areas" identified in this figure are the result of a spatial analysis performed to eliminate those areas at RMA deemed "unsuitable" with respect to the primary and secondary landfill siting criteria. The primary landfill siting criteria include avoidance of wetlands sensitive habitats, 100-year floodplain, organic groundwater contamination plumes, and human health exceedance areas as listed in Table 4.2. The secondary landfill siting criteria are identified in the box titled secondary landfill siting criteria.

- Suitable areas based on primary landfill siting criteria
- Suitable areas based on secondary landfill siting criteria
- Road
- Section number
- Northwest Boundary Containment System
- Submerged Quench Incinerator

Total suitable acres based on primary and secondary landfill siting criteria, 233
Maximum contiguous suitable acres 217

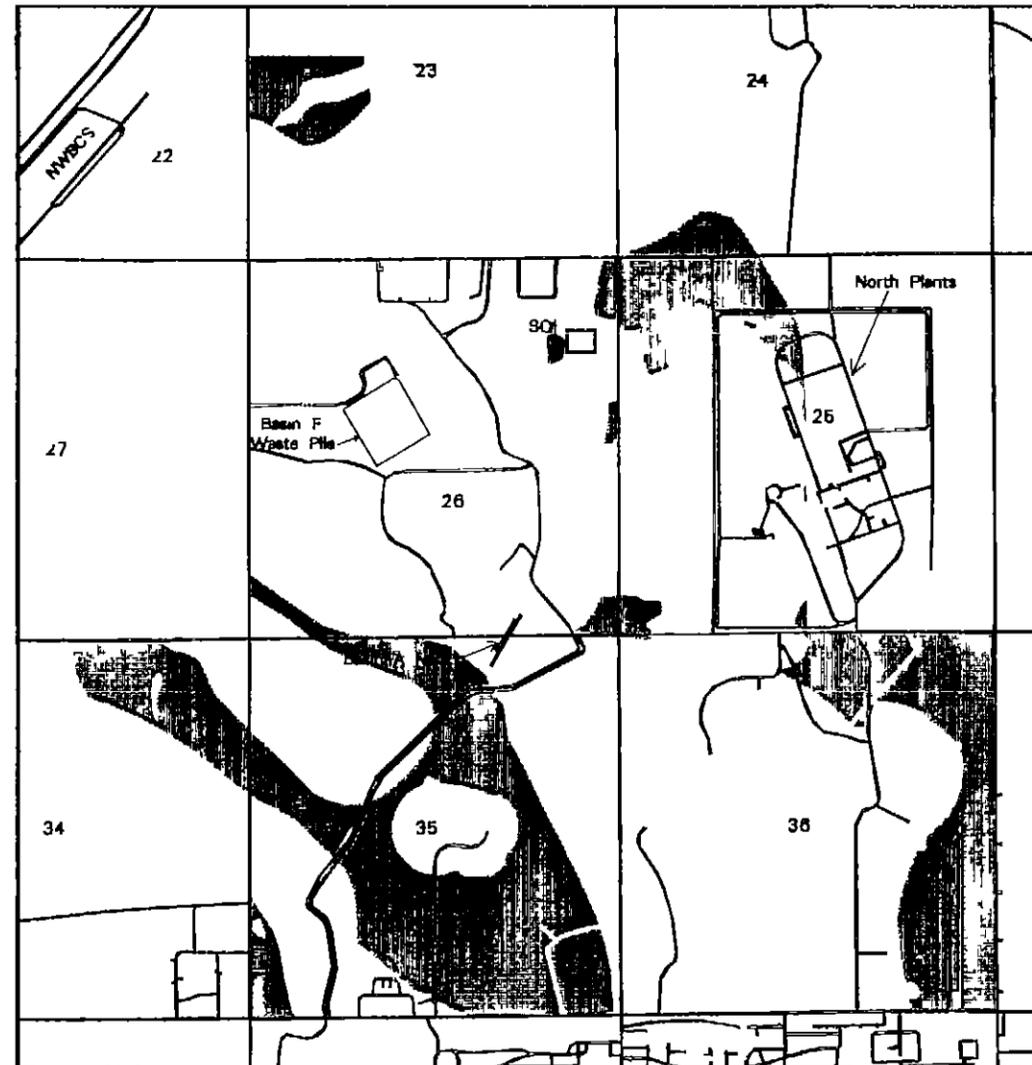


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Figure 4.15 - Applied Selection Criteria - Depth to Groundwater Greater Than or Equal to 50 feet, and Depth to Bedrock Less Than or Equal to 30 feet

Potential Landfill Areas



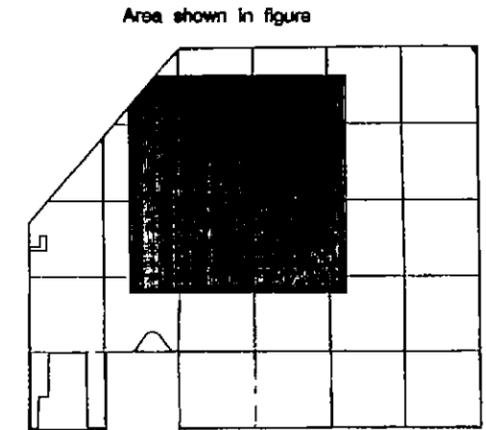
Secondary Landfill Siting Criteria

The values listed below were used to determine siting potential based on depth to groundwater, depth to bedrock, and depth to saturated alluvium.

Maximize	
Depth to groundwater (in feet)	
10 to 50 feet	Not suitable
50 to 90 feet	Suitable
Minimize	
Depth to bedrock (in feet)	
0 to 40 feet	Suitable
40 to 80 feet	Not Suitable

Note: Secondary landfill siting criteria includes unsaturated alluvium.

- Suitable areas based on primary landfill siting criteria
- Suitable areas based on secondary landfill siting criteria
- Road
- 12 Section number
- NWBCS Northwest Boundary Containment System
- SOI Submerged Quench Incinerator



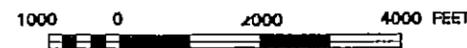
Rocky Mountain Arsenal

Note
The "Suitable Areas" identified in this figure are the result of a spatial analysis performed to eliminate those areas at RMA deemed "unsuitable" with respect to the primary and secondary landfill siting criteria. The primary landfill siting criteria include avoidance of wetlands, sensitive habitats, 100-year floodplain, organic groundwater contamination plumes, and human health exceedance areas as listed in Table 4.2. The secondary landfill siting criteria are identified in the box titled secondary landfill siting criteria.

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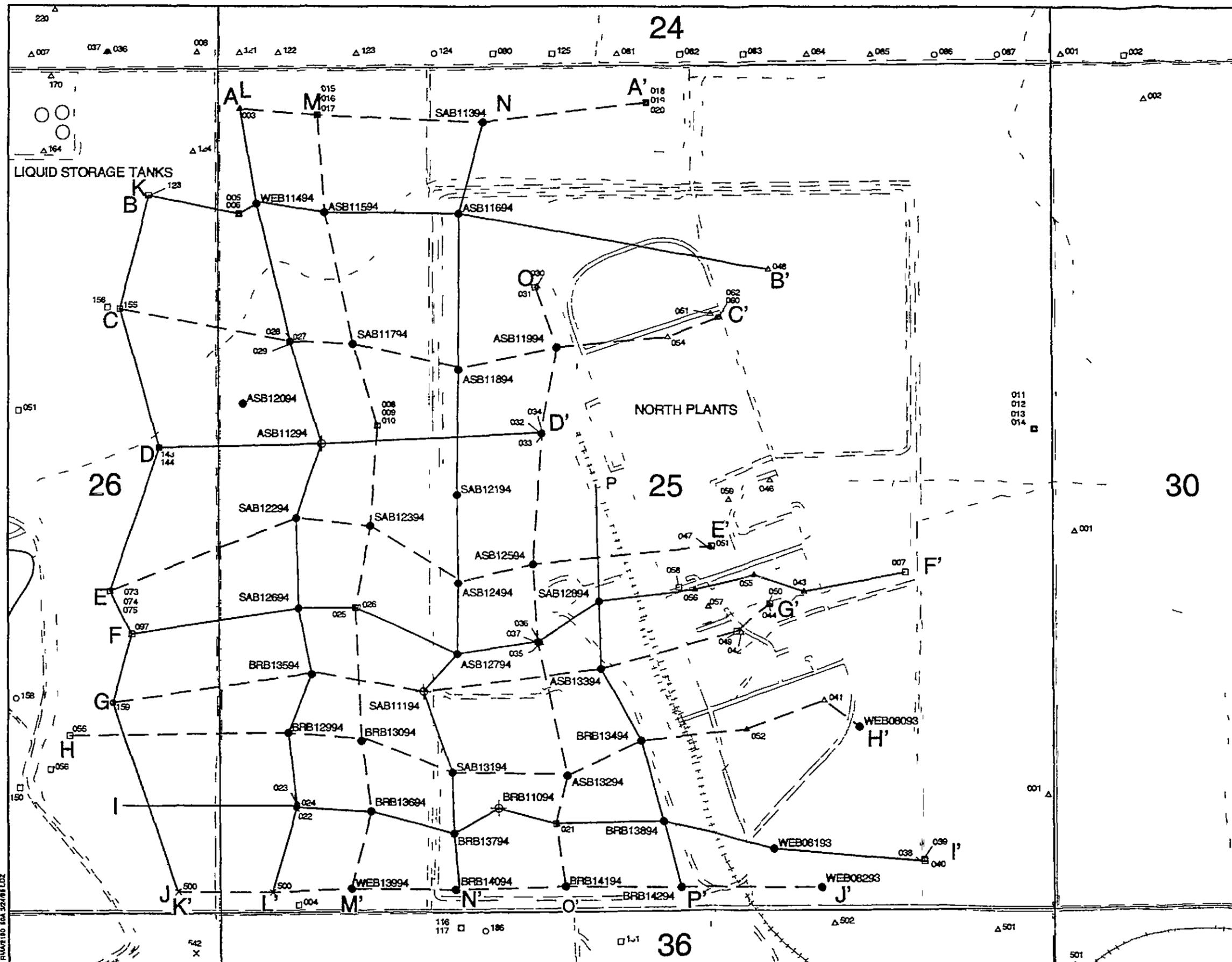
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Total suitable acres based on primary and secondary landfill siting criteria: 325
Maximum contiguous suitable acres 299



Scale 1:30,000
One inch represents 2,500 feet

Figure 4.16 - Applied Selection Criteria - Depth to Groundwater Greater Than or Equal to 50 feet, and Depth to Bedrock Less Than or Equal to 40 feet



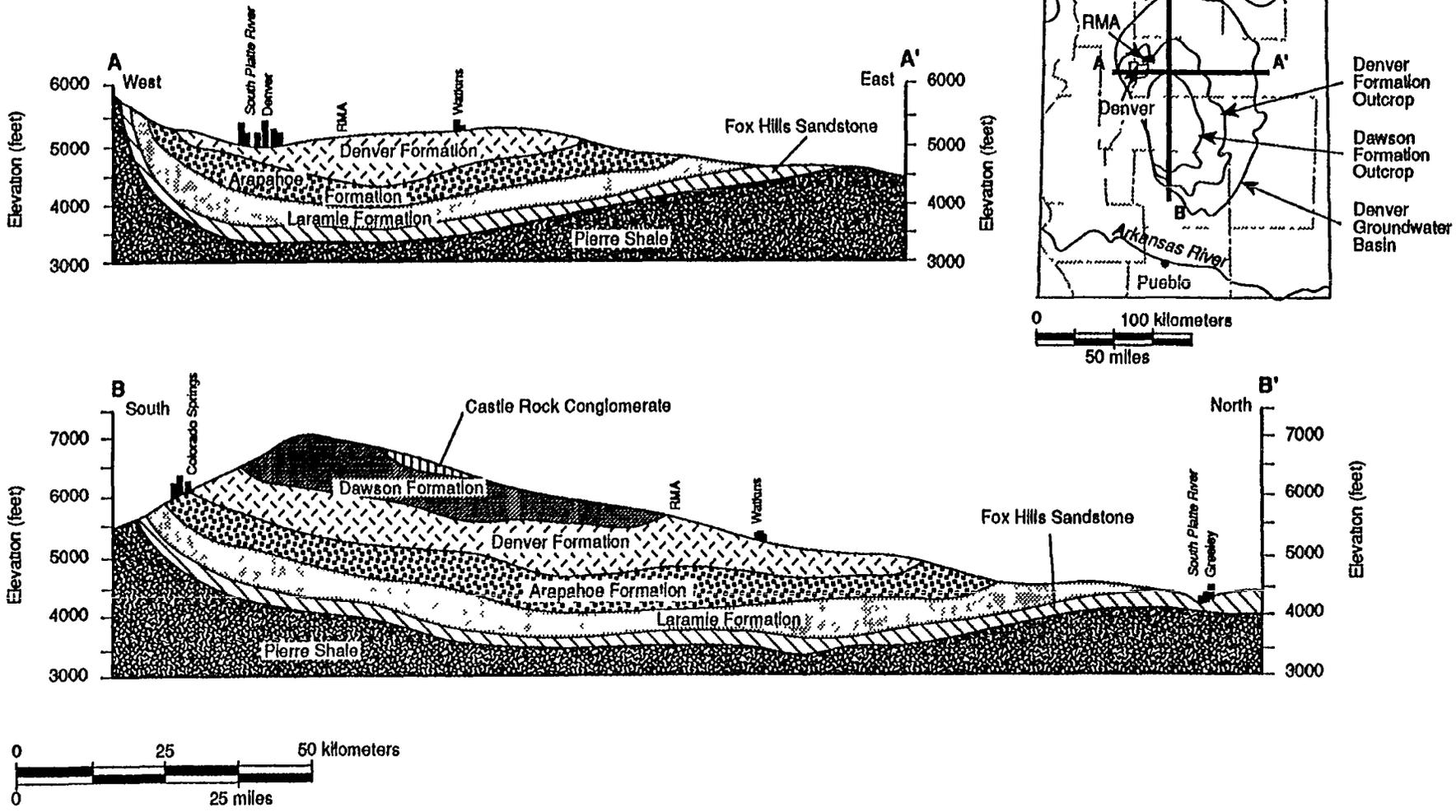
Explanation	
△ 038	Alluvial well with well number
○ 192	Unconfined Denver Formation well with well number
□ 123	Confined Denver Formation well with well number
● WEB11494	Shallow Soil Borings (30)
⊕ BRB11094	Deep Bore Holes (3)
---	Unpaved roadway
- - -	Surface drainage
25	Section number
B — B'	Cross sections developed for evaluation of geologic data (included in report)
A - - A'	Cross sections developed for evaluation of geologic data (not included in report)

Scale in feet

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Figure 4 17
 Boring and Cross-Section Location Map
 Section 25

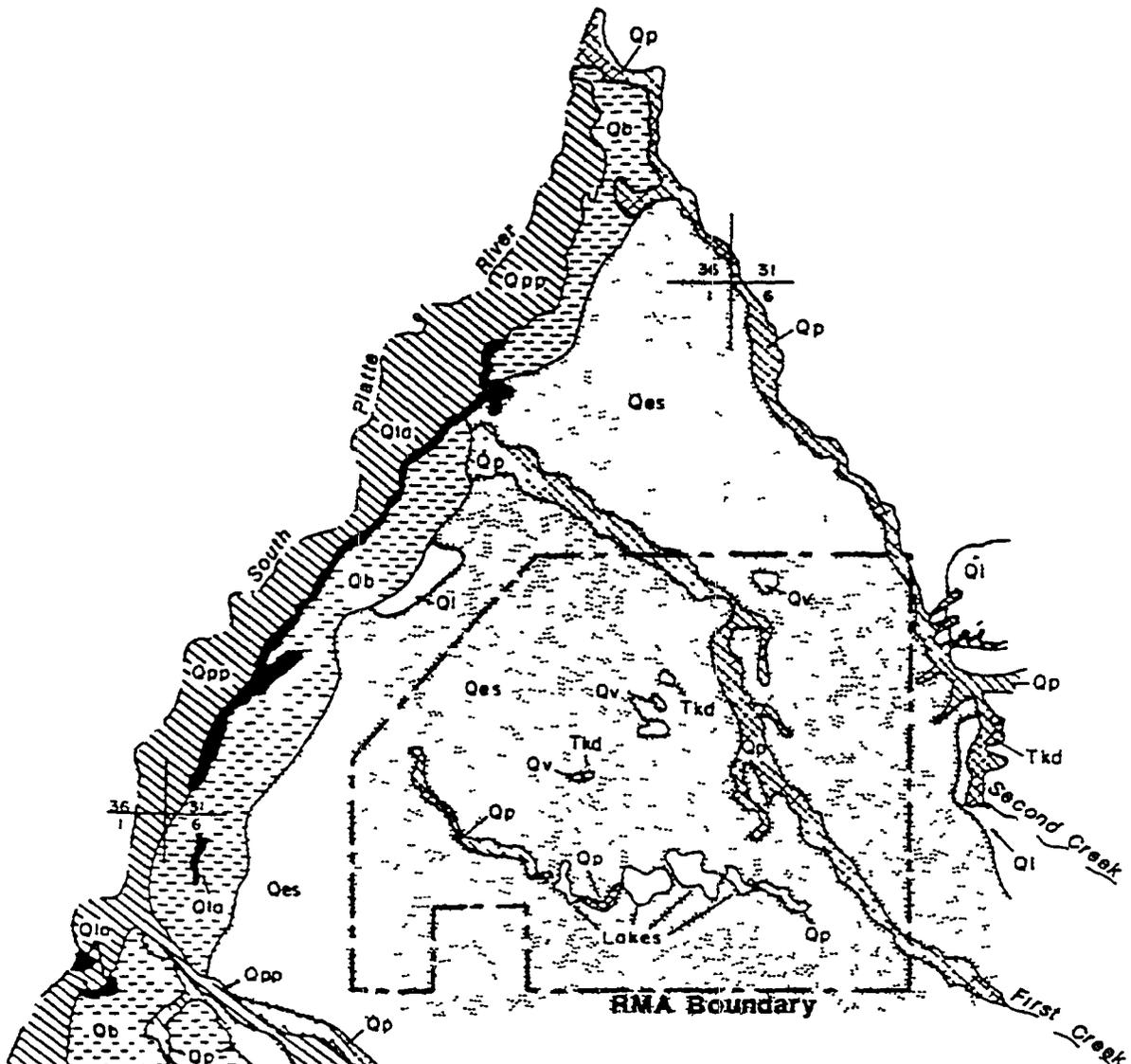


Source Robson and Romero, 1981

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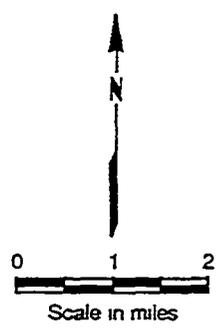
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Figure 4 18
 Upper Stratigraphic Sections of the Denver Basin



EXPLANATION

- Post Piney Creek Alluvium (Quaternary)
- Piney Creek Alluvium (Quaternary)
- Eolian Sand (Quaternary)
- Loess (Quaternary)
- Broadway Alluvium (Quaternary)
- Louviers Alluvium (Quaternary)
- Verdas Alluvium (Quaternary)
- Denver Formation (Upper Cretaceous & Tertiary)

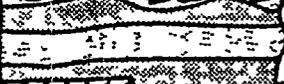
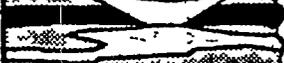


Source Lindvall 1980

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Figure 4 19
 Surficial Geologic Map of the
 Rocky Mountain Arsenal Area

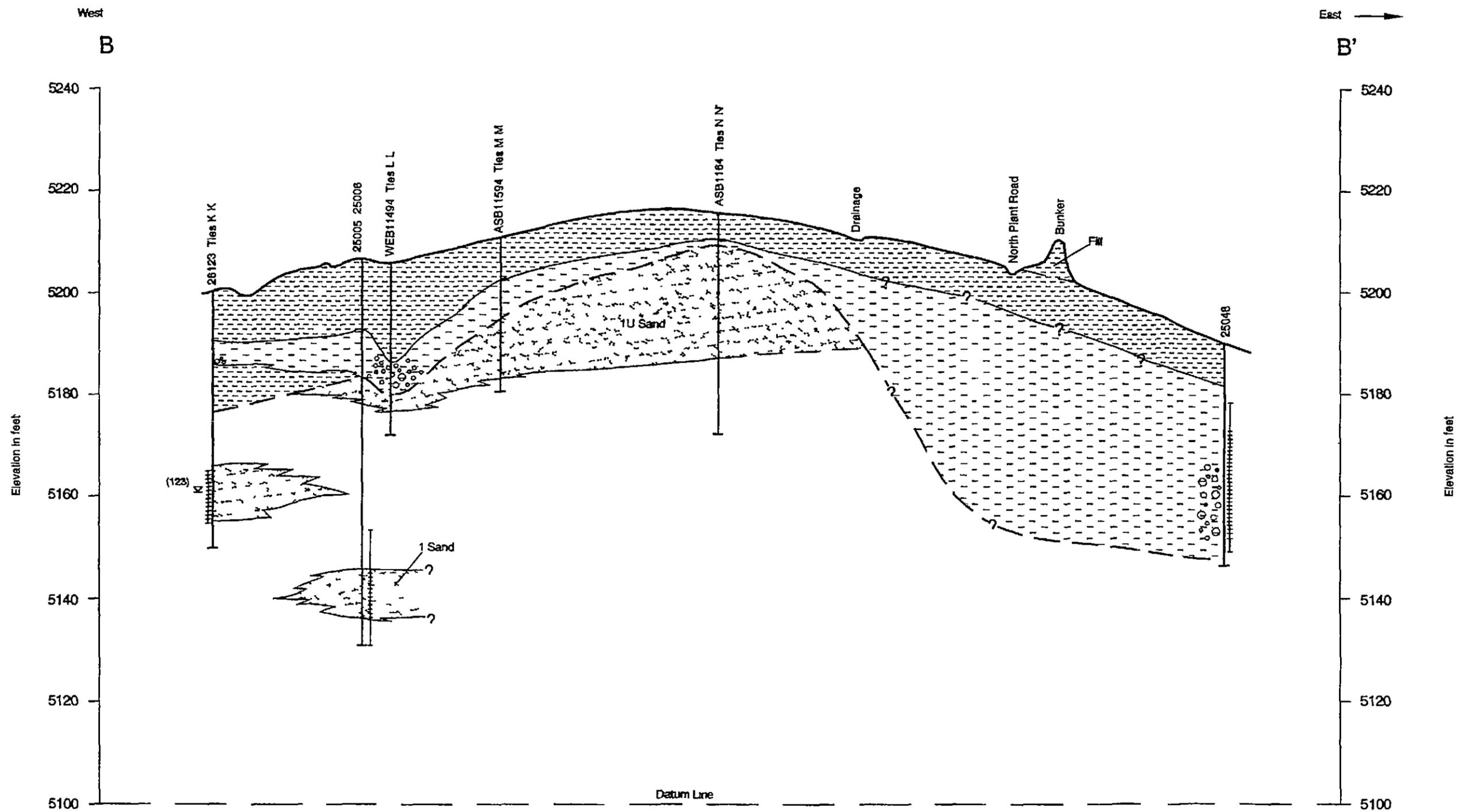
System	Zone and Thickness		Lithologic Description		
Quaternary	Alluvium	0 - 120		Gravel, silty sand, sandy silt, and clay, laterally variable	
	B	0 - 25' (0 - 19')		B Sand, sandstone, and claystone	
Cretaceous - Tertiary Denver Formation	Volcaniclastic	0 - 50'		Volcaniclastic material and laterally equivalent claystone and sandstone	
	A	0 - 75'		A Upper (AU) (0 - 13')	Sandstone, claystone, and lignite
				A Middle (AM) (0 - 20')	
				A Lower (AL) (0 - 20')	
	A Sand (0 - 46')				
	Lignite A	0 - 11'			
	IU	0 - 40' (0 - 36')			
	Lignite B	0 - 12'			
	1	0 - 60' (0 - 54')			
	Lignite C	0 - 13'			
	2	0 - 55' (0 - 41')			
	Lignite D	0 - 13'			
	3	0 - 45' (0 - 33')			Volcaniclastic material and laterally equivalent claystone and sandstone
	4	0 - 50' (0 - 36')			
	5	0 - 25' (0 - 19')			
6	0 - 30' (0 - 23')				
7	0 - 30' (0 - 20')				
8	0 - 40' (0 - 27')				
9	0 - 20' (0 - 6')				

Source Environmental Science and Engineering 1988

Note Thickness not to scale, net sandstone thickness shown in parentheses

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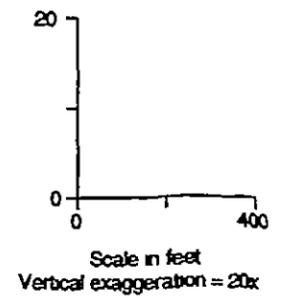
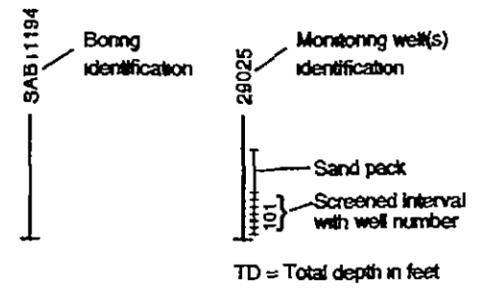
Figure 4 20
 Denver Formation Stratigraphic Column



EXPLANATION

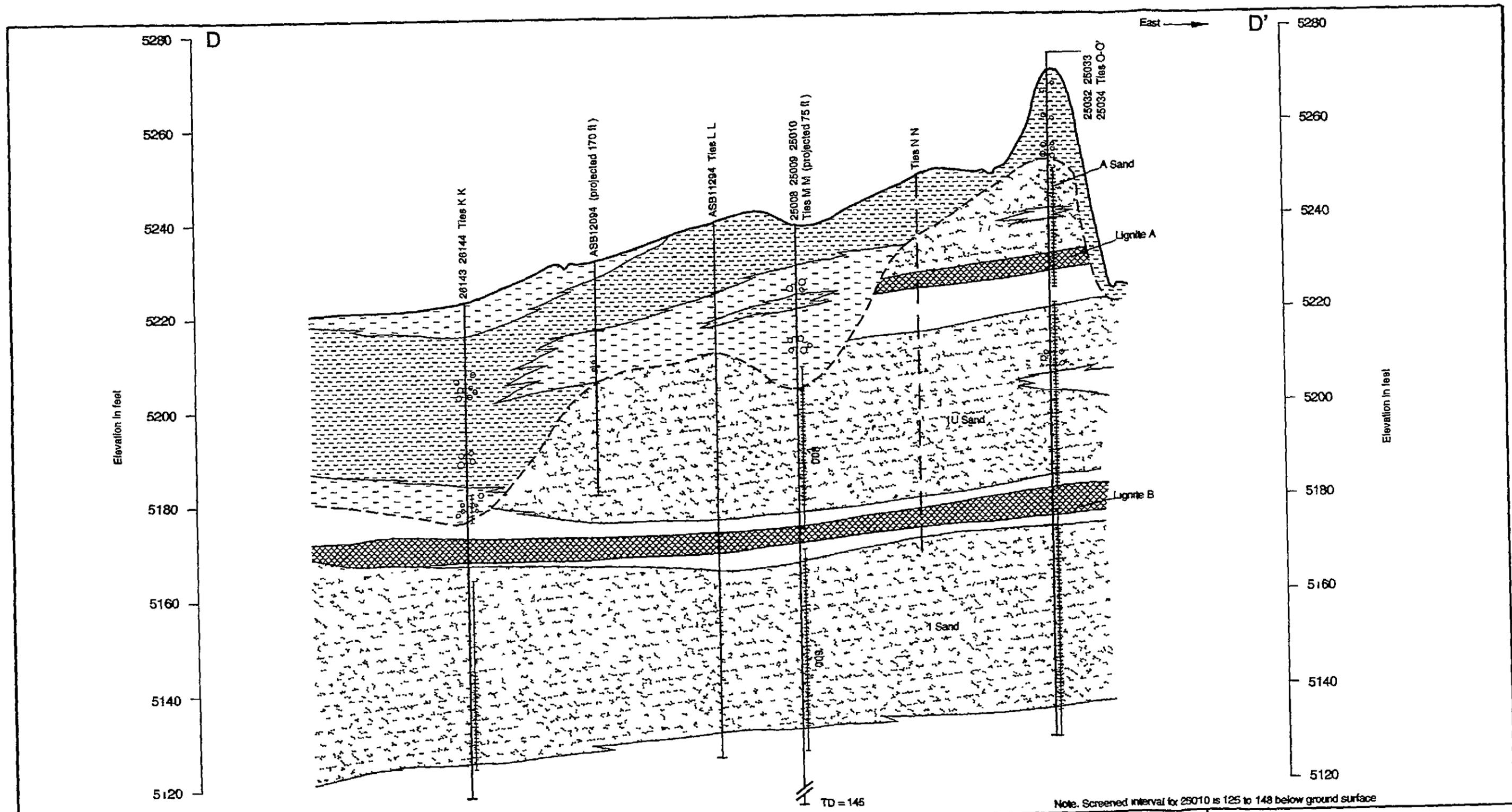
- | | | | |
|------------------|---|------------------------------------|---|
| <u>Alluvium:</u> | | <u>Bedrock (Denver Formation):</u> | |
| | Clay sandy clay (CL, CH) | | Lignite/Lignitic claystone |
| | Sand silty sand, clayey sand (SP, SM, SC, SW) | | Sandstone |
| | Gravel | | Claystone with interbedded siltstone, lignite, and sandstone (see note) |
- Top of weathered bedrock

Note:
In areas of no lithologic control lateral extent of Denver Formation sandstone, lignite, and claystone is unknown.



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Figure 4.21
Geologic Cross Section B-B



Note. Screened interval for 25010 is 125 to 148 below ground surface

EXPLANATION

Alluvium:

- Clay sandy clay (CL, CH)
- Sand, silty sand, clayey sand (SP, SM, SC, SW)
- Gravel

Bedrock (Denver Formation):

- Lignite/Lignitic Claystone
- Sandstone
- Claystone with interbedded siltstone, lignite, and sandstone (see note)

--- Top of weathered bedrock

Note:
In areas of no lithologic control lateral extent of Denver Formation sandstone, lignite and claystone is unknown.

SAB11194

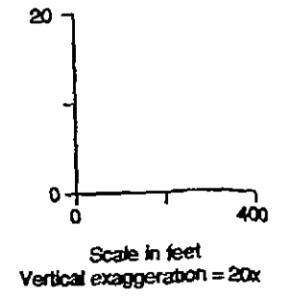
Boring identification

28025

Monitoring well(s) identification

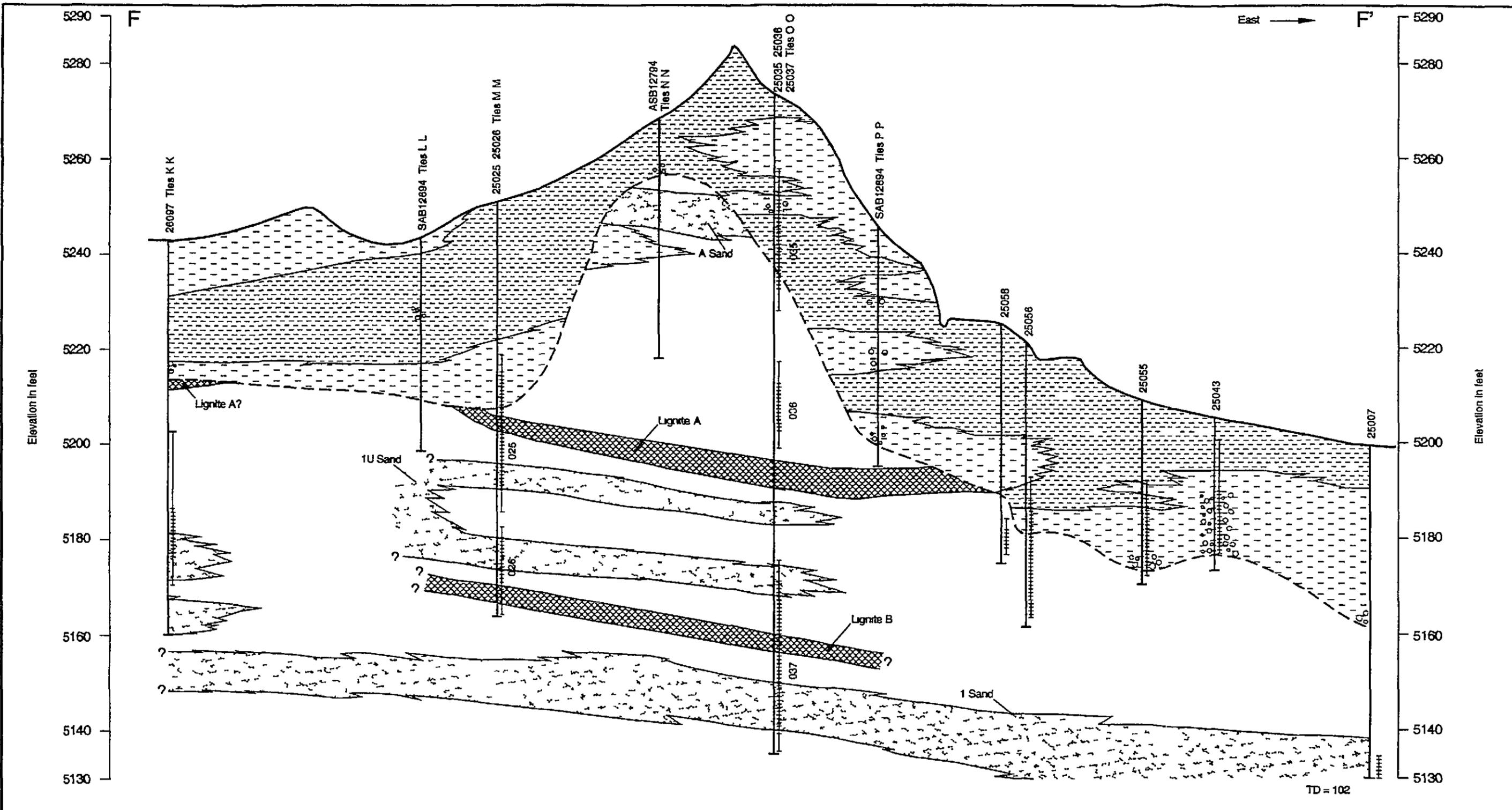
Sand pack
Screened interval with well number

TD = Total depth in feet



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Figure 4.22
Geologic Cross Section D-D'

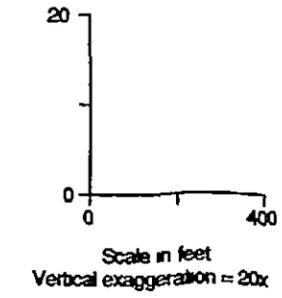
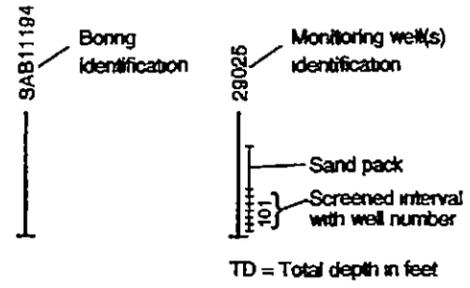


EXPLANATION

- | | |
|--|---|
| Alluvium | Bedrock (Denver Formation) |
| Clay sandy clay (CL, CH) | Lignite/Lignitic claystone |
| Sand, silty sand, clayey sand (SP, SM, SC, SW) | Sandstone |
| Gravel | Claystone with interbedded siltstone, lignite, and sandstone (see note) |

--- Top of weathered bedrock

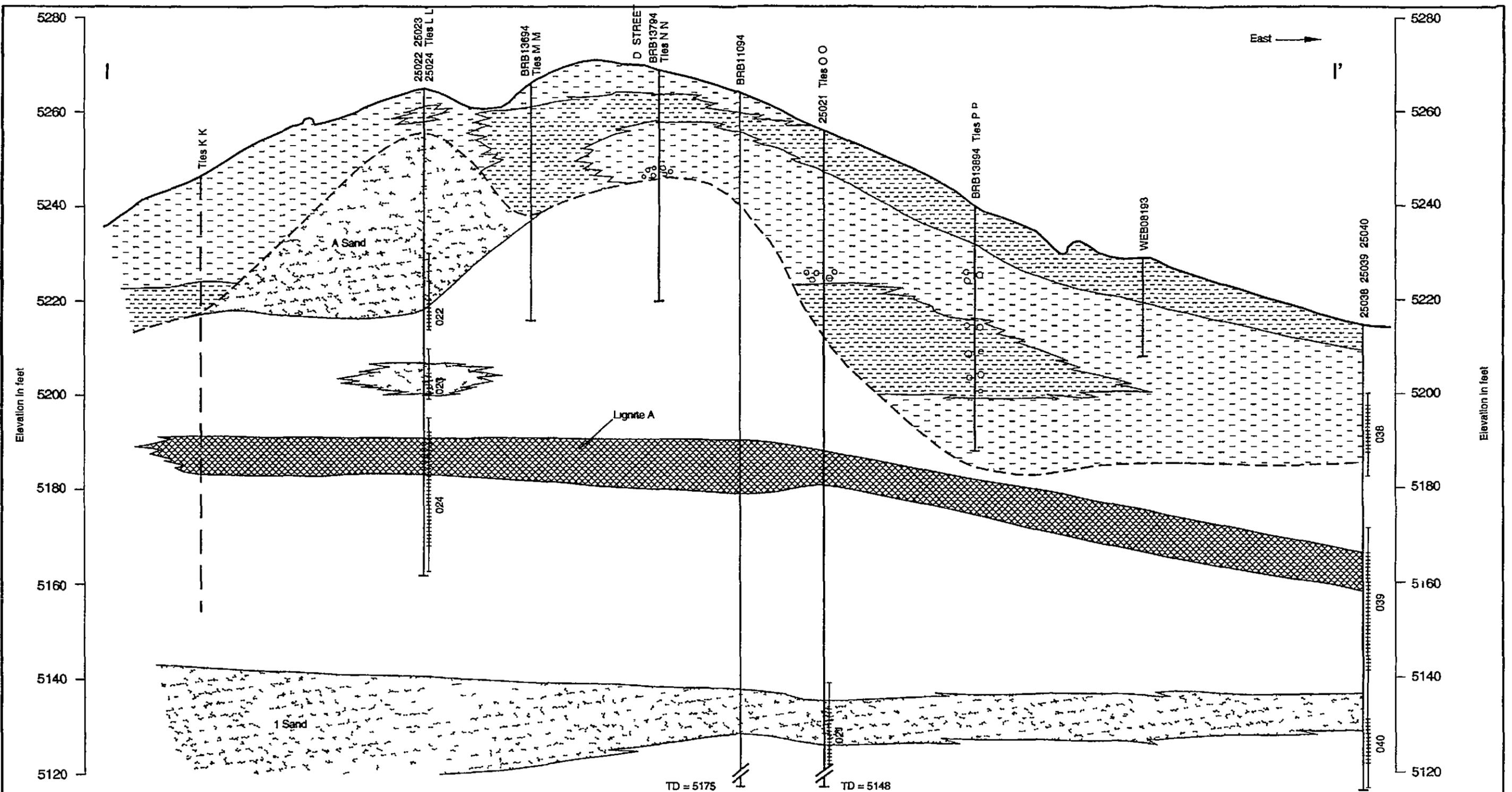
Note:
In areas of no lithologic control lateral extent of Denver Formation sandstone, lignite, and claystone is unknown.



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Figure 4.23

Geologic Cross Section F-F'

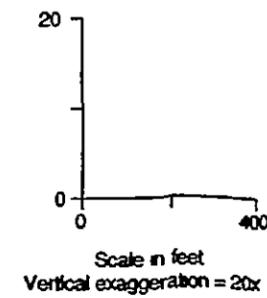


EXPLANATION

- | | | | |
|------------------|--|------------------------------------|---|
| Alluvium: | | Bedrock (Denver Formation): | |
| | Clay sandy clay (CL, CH) | | Lignite/Lignitic claystone |
| | Sand, silty sand, clayey sand (SP, SM, SC, SW) | | Sandstone |
| | Gravel | | Claystone with interbedded siltstone, lignite, and sandstone (see note) |
| | | | Top of weathered bedrock |

Note:
In areas of no lithologic control lateral extent of Denver Formation sandstone, lignite and claystone is unknown

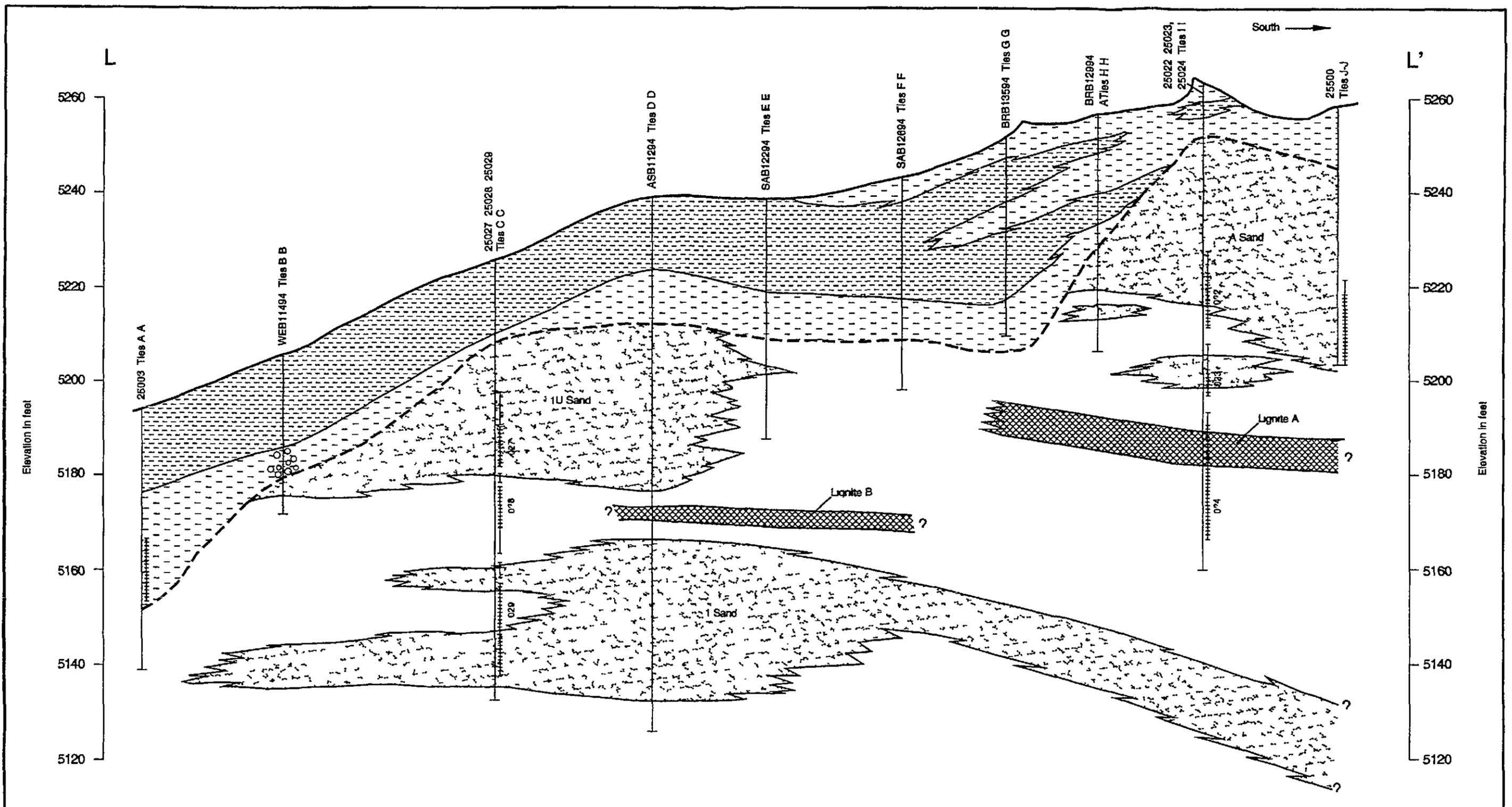
- | | | | |
|--------------------------|-----------------------|--|------------------------------------|
| | Boring identification | | Monitoring well(s) identification |
| | Sand pack | | Screened interval with well number |
| TD = Total depth in feet | | | |



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Figure 4.24
Geologic Cross Section I-I'

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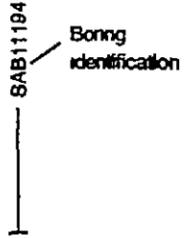
EXPLANATION

- Alluvium:**
- Clay sandy clay (CL, CH)
 - Sand, silty sand, clayey sand (SP, SM, SC, SW)
 - Gravel

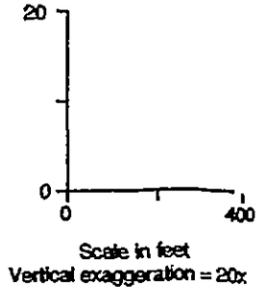
- Bedrock (Denver Formation)**
- Lignite/Lignitic claystone
 - Sandstone
 - Claystone with interbedded siltstone, lignite, and sandstone (see note)

Top of weathered bedrock

Note:
In areas of no lithologic control lateral extent of Denver Formation sandstone, lignite, and claystone is unknown

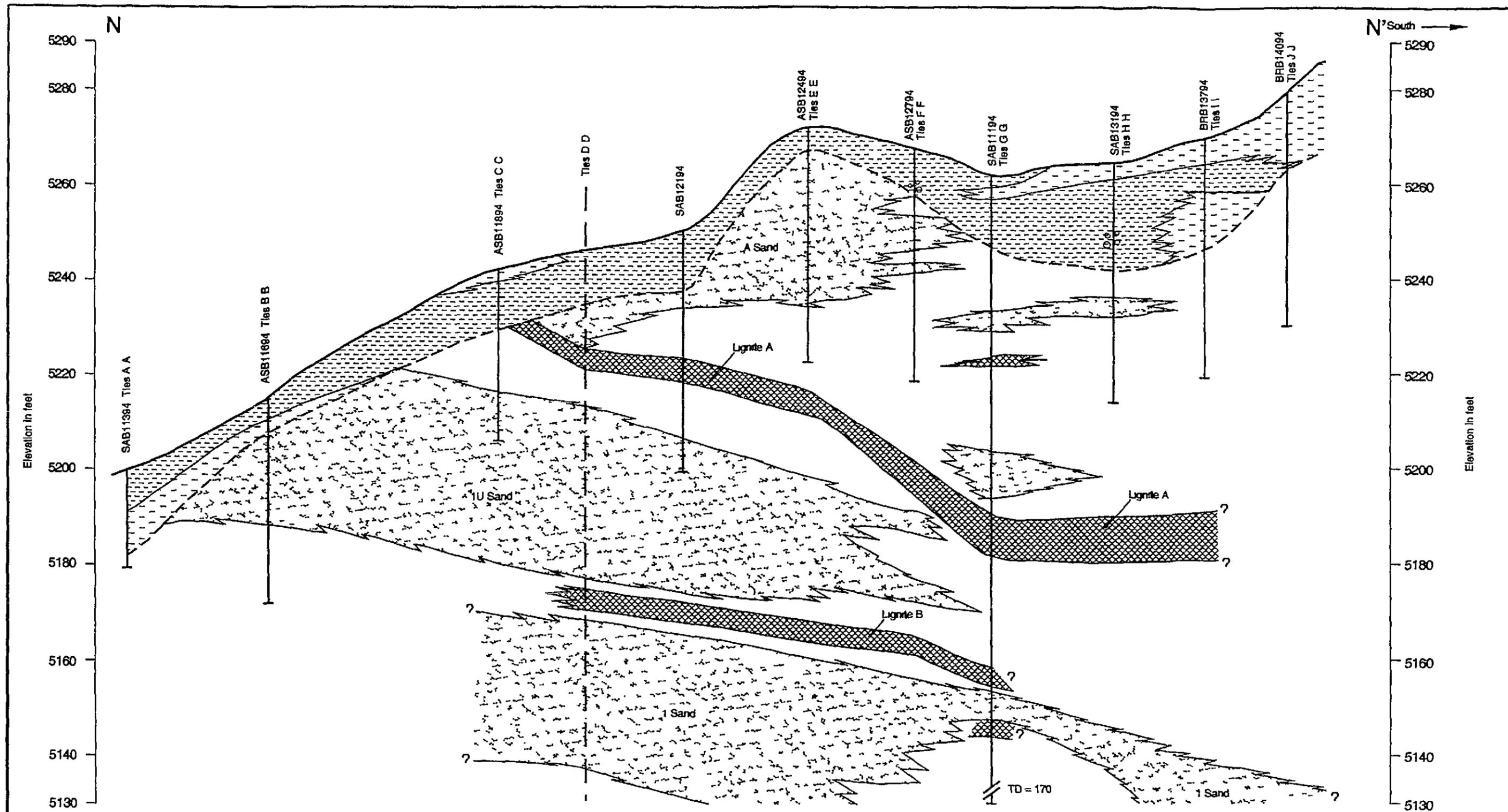


Monitoring well(s) identification
Sand pack
Screened interval with well number
TD = Total depth in feet



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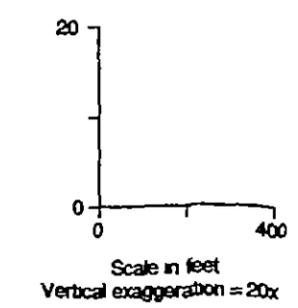
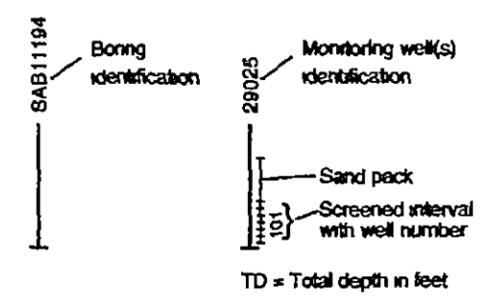
Figure 4.26
Geologic Cross Section L-L



EXPLANATION

- | | | | |
|--|---|------------------------------------|--------------------------|
| <u>Alluvium:</u> | | <u>Bedrock (Denver Formation):</u> | |
| Clay sandy clay (CL, CH) | Lignite/Lignitic claystone | Sandstone | Top of weathered bedrock |
| Sand, silty sand, clayey sand (SP, SM, SC, SW) | Claystone with interbedded siltstone, lignite, and sandstone (see note) | | |
| Gravel | | | |

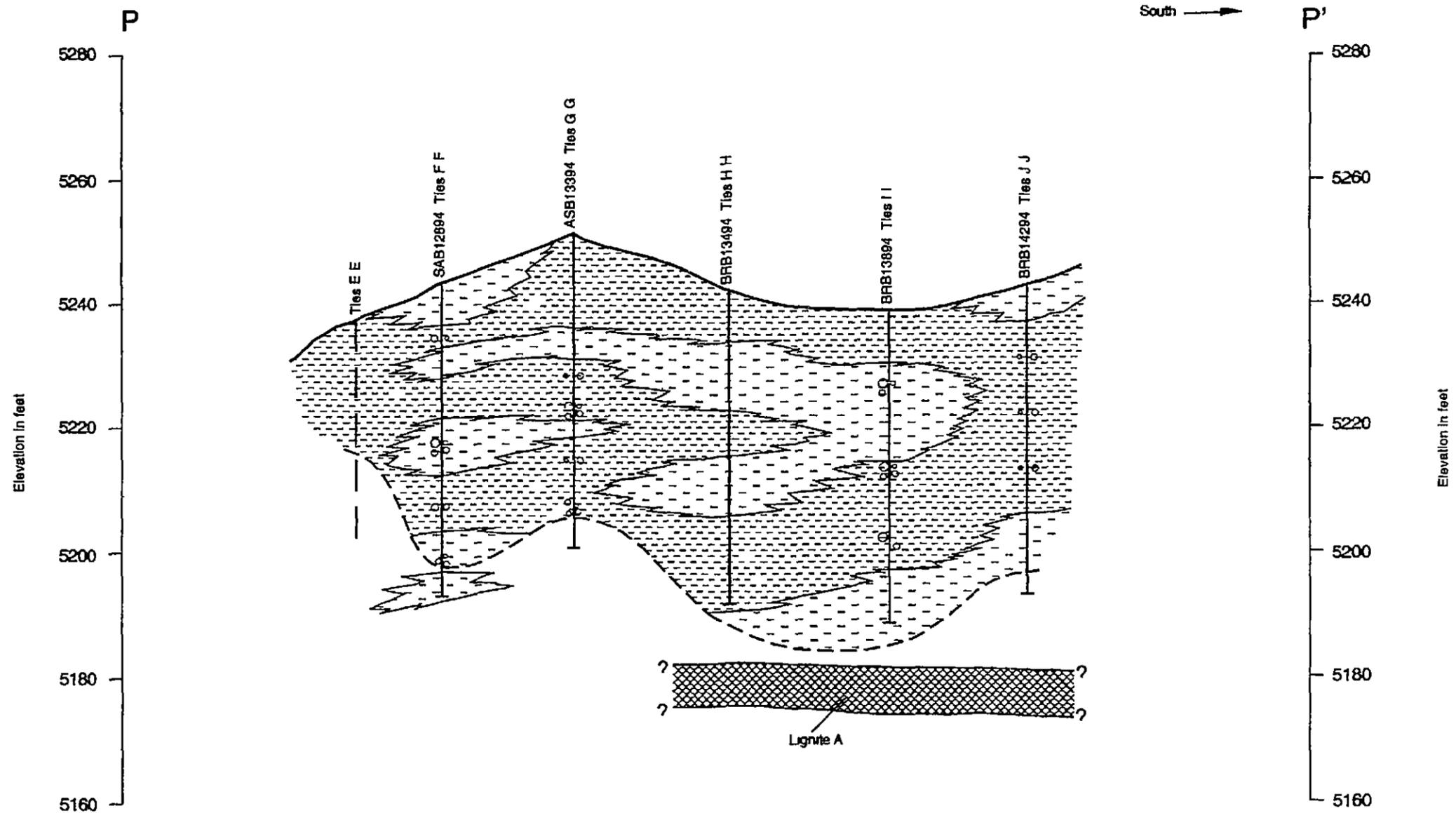
Note:
In areas of no lithologic control lateral extent of Denver Formation sandstone, lignite, and claystone is unknown.



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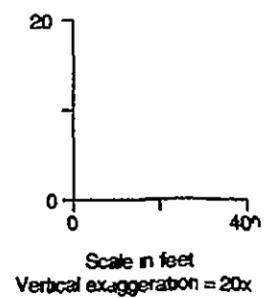
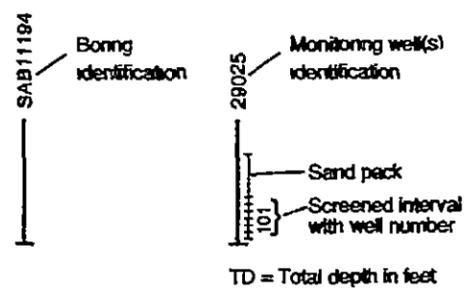
Figure 4.27

Geologic Cross Section N-N'



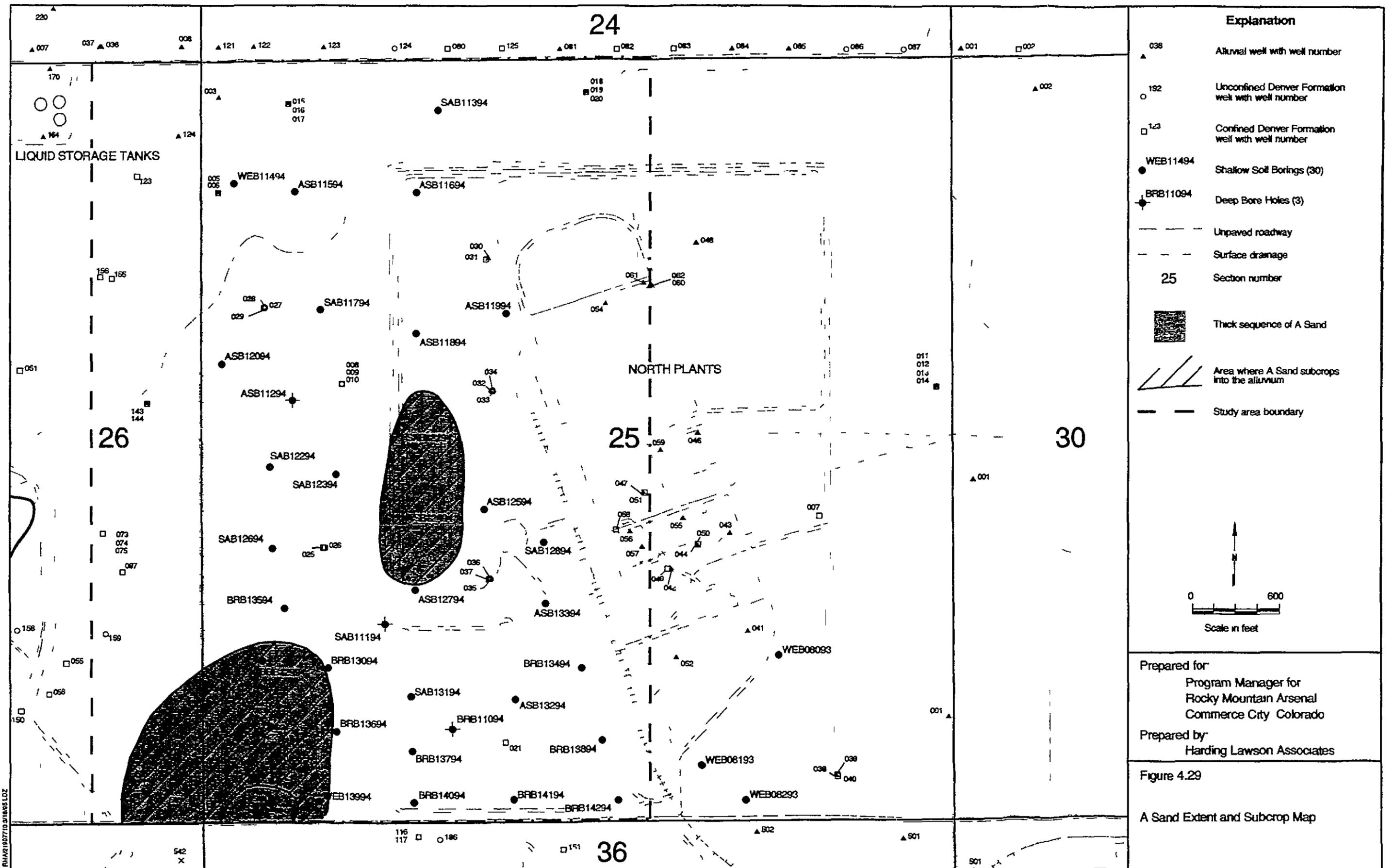
EXPLANATION	
Alluvium:	Bedrock (Denver Formation):
Clay sandy clay (CL, CH)	Lignite/Lignitic claystone
Sand silty sand clayey sand (SP, SM, SC, SW)	Sandstone
Gravel	Claystone with interbedded siltstone, lignite, and sandstone (see note)
	Top of weathered bedrock

Note:
In areas of no lithologic control lateral extent of Denver Formation sandstone, lignite, and claystone is unknown.

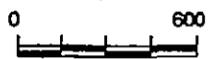


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Figure 4.28
Geologic Cross Section P-P'



Explanation	
▲ 038	Alluvial well with well number
○ 132	Unconfined Denver Formation well with well number
□ 123	Confined Denver Formation well with well number
● WEB11494	Shallow Soil Borings (30)
◆ BRB11094	Deep Bore Holes (3)
---	Unpaved roadway
---	Surface drainage
25	Section number
■	Thick sequence of A Sand
▨	Area where A Sand subcrops into the alluvium
---	Study area boundary

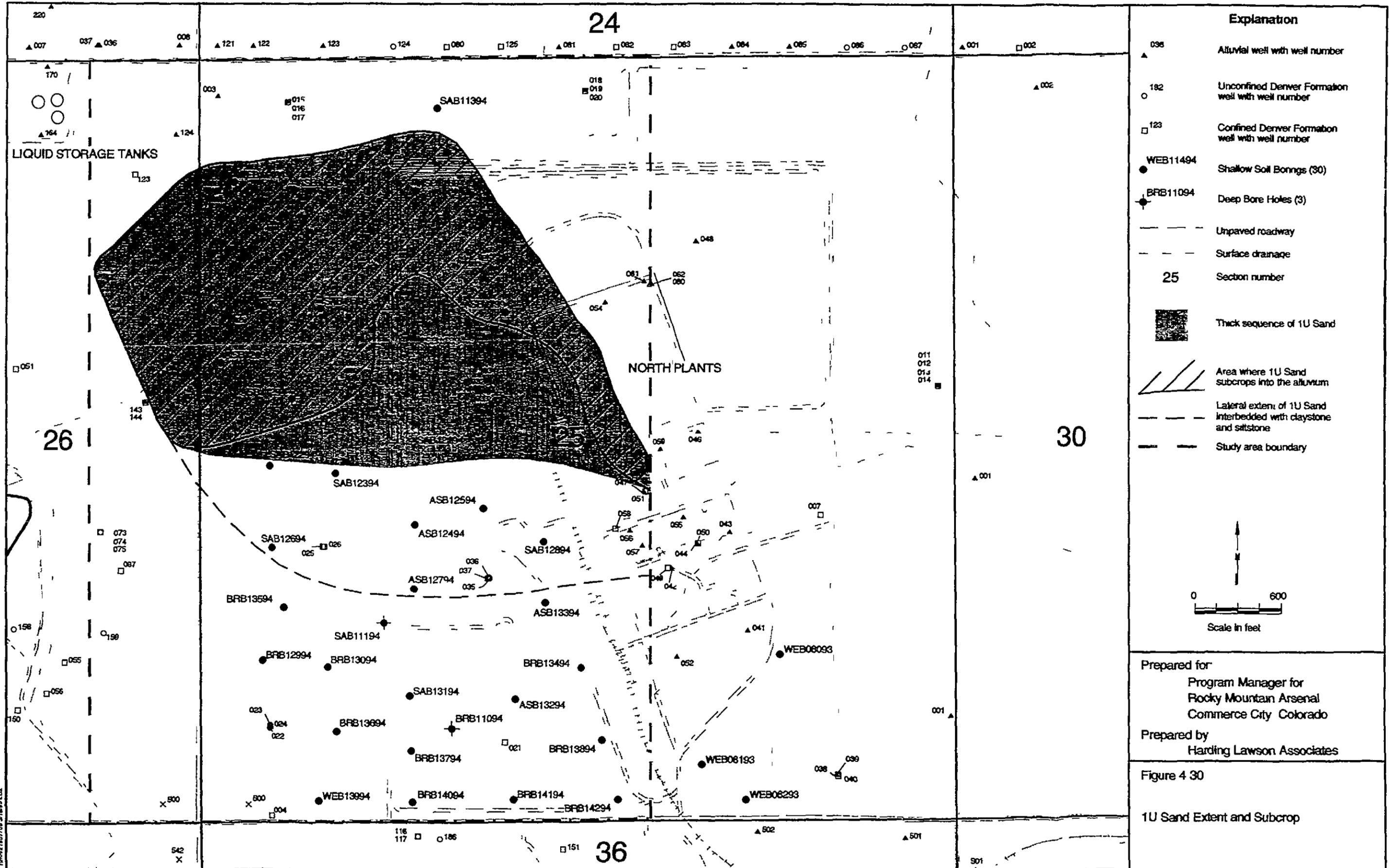

 Scale in feet


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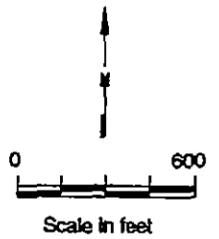
Figure 4.29
 A Sand Extent and Subcrop Map

RMAA21007710.3/16005.LDZ



Explanation

- ▲ 036 Alluvial well with well number
- 182 Unconfined Denver Formation well with well number
- 123 Confined Denver Formation well with well number
- WEB11494 Shallow Soil Borings (30)
- ⊙ BRB11094 Deep Bore Holes (3)
- Unpaved roadway
- - - Surface drainage
- 25 Section number
- Thick sequence of 1U Sand
- ▨ Area where 1U Sand subcrops into the alluvium
- - - Lateral extent of 1U Sand interbedded with claystone and siltstone
- Study area boundary

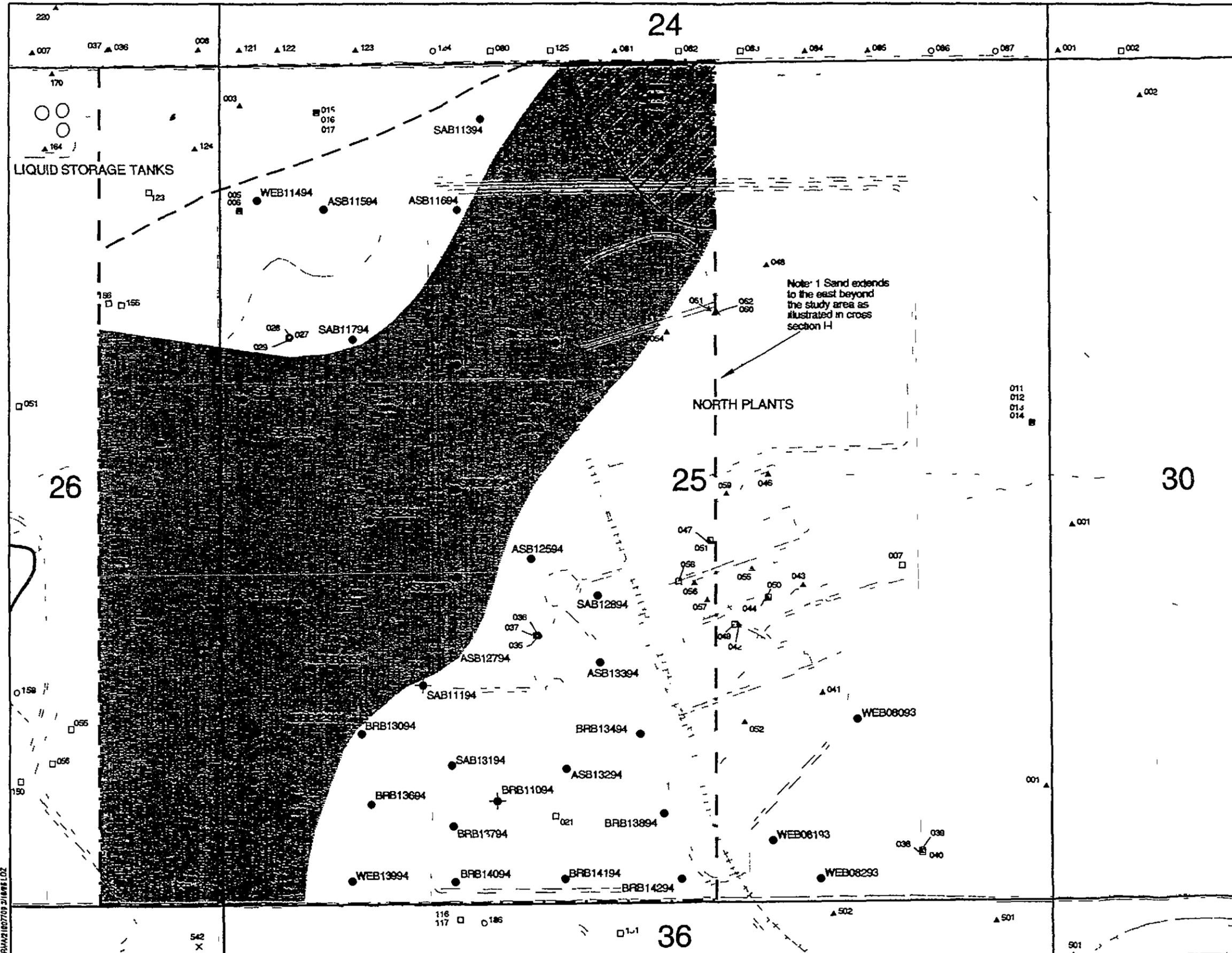


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Figure 4 30
 1U Sand Extent and Subcrop

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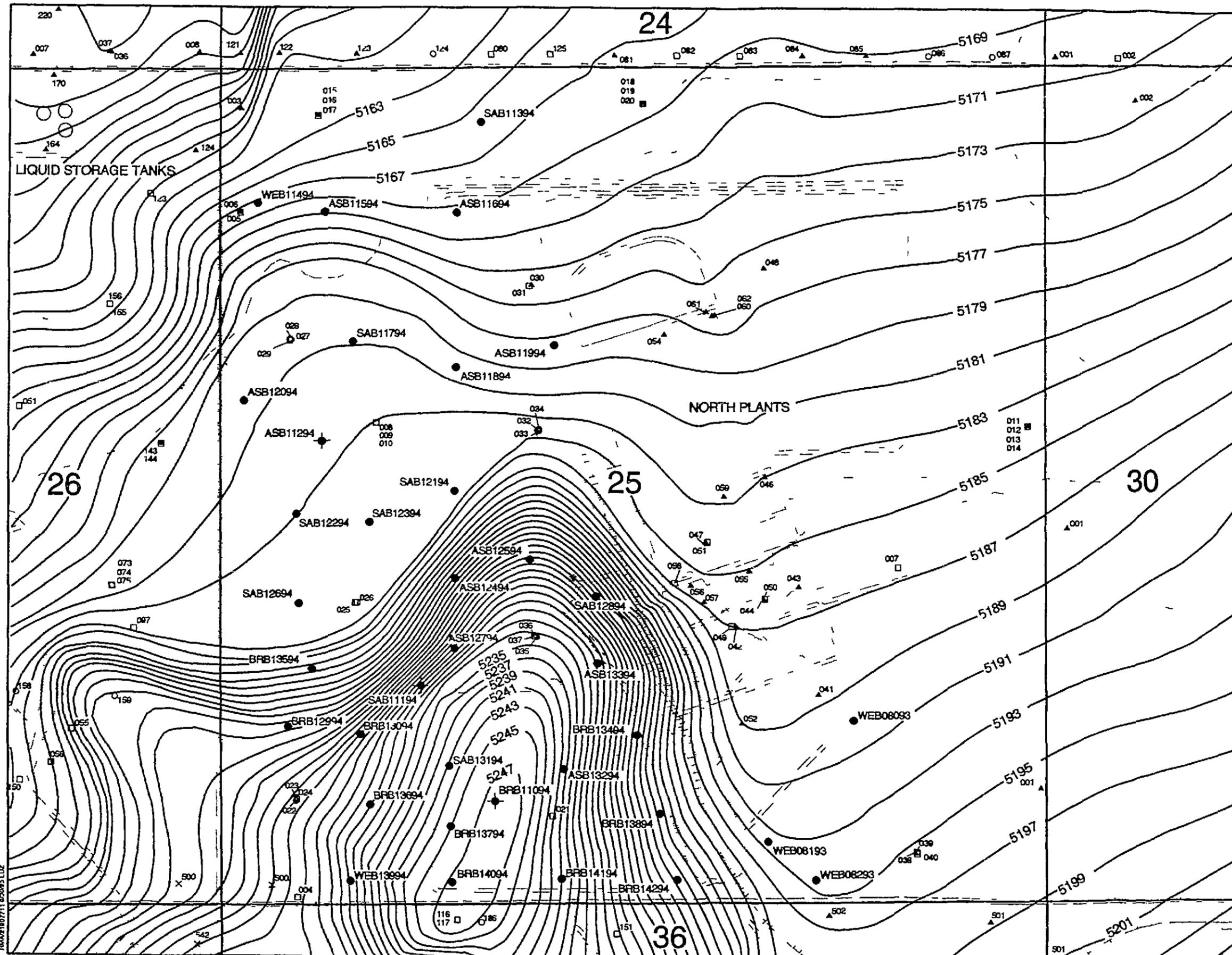


Explanation	
▲ 038	Alluvial well with well number
○ 192	Unconfined Denver Formation well with well number
□ 123	Confined Denver Formation well with well number
● WEB11494	Shallow Soil Borings (30)
✦ BRB11094	Deep Bore Holes (3)
---	Unpaved roadway
- - -	Surface drainage
25	Section number
[Stippled Area]	Thick sequence of 1 Sand
[Diagonal Lines]	Areas where 1 Sand subcrops into the alluvium
- - -	Lateral extent of 1 Sand interbedded with claystone and siltstone
---	Study area boundary

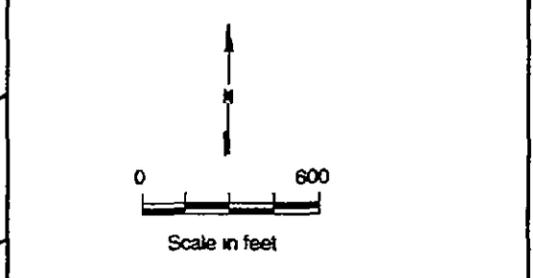
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Figure 4 31
 1 Sand Extent and Subcrop



Explanation	
038	Artesian well with well number
192	Unconfined Denver Formation well with well number
123	Confined Denver Formation well with well number
WEB11494	Shallow Soil Borings (30)
BRB11094	Deep Bore Holes (3)
---	Unpaved roadway
- - -	Surface drainage
25	Section number

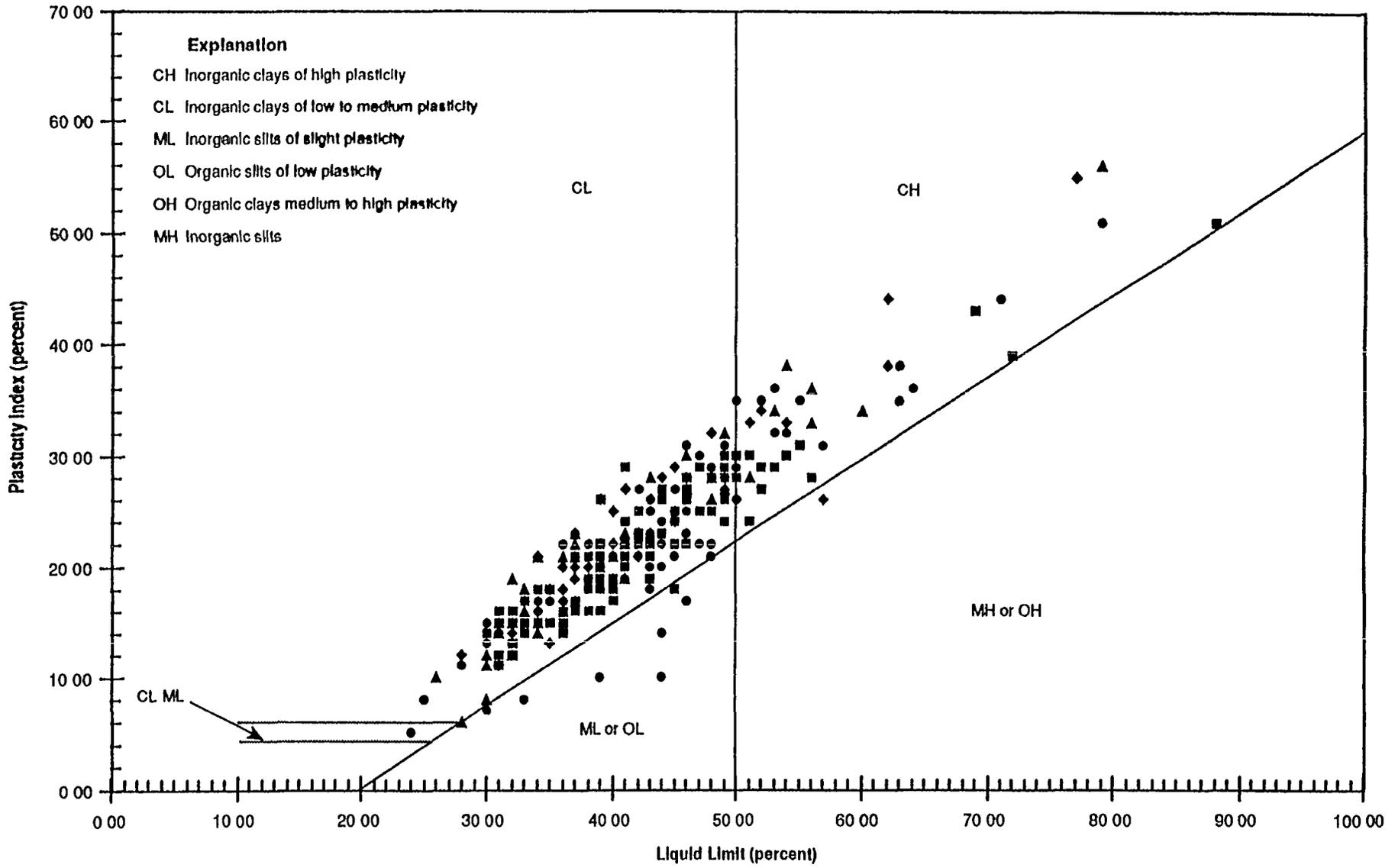


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Figure 4 32
 Water-Level Contours

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Figure 4.33
Plasticity Chart

5.0 SITE FEASIBILITY STUDY

This section presents the results of the Site Feasibility Study (FS). The primary objectives of the Site FS were to identify an appropriate landfill site within the area identified in the Area FS and to provide information on conceptual landfill models, design elements, construction costs, and schedules. The Site FS links the data from the Material and Area Feasibility Studies to evaluate the site-specific requirements of an onpost landfill that will use onpost soils for the soil component of caps and liners, have sufficient capacity to dispose of anticipated waste volumes, and meet applicable federal, state, and local regulatory requirements.

The study included a review and evaluation of the following criteria:

- Waste types, volumes, and generation rates
- Regulatory criteria
- Specific site considerations and limitations
- Conceptual landfill design alternatives
- Evaluation and screening of alternatives
- Facility layout
- Material quantities and availability
- Construction cost estimates and schedules
- Operations and maintenance requirements

5.1 Waste Data

Waste data is a critical component of any landfill design. The amount of waste requiring disposal dictates the size of the landfill needed. The type of waste impacts placement, compaction, and cover criteria. The rate at which the waste is generated affects operation and maintenance criteria and construction phasing. The following section discusses:

- Projected waste types that may be disposed of at an onsite hazardous waste landfill

- Estimated waste volumes
- Possible waste generation rates

The data presented are based on information obtained in the Proposed Final Detailed Analysis of Alternatives (DAA) (Ebasco, 1994) as summarized by RUST Environment and Infrastructure (RUST E&I, 1994)

5.1.1 Projected Waste Types and Characteristics

Based on information in the DAA, the potentially contaminated materials at the RMA that may be landfilled can be classified into three waste types: hazardous and toxic materials, unexploded ordnance (UXO), and surety-contaminated materials. The two major waste forms within these waste types include contaminated soils and building debris.

The actual waste volumes generated for landfilling will be dependant on the selected remedial action alternative. The alternatives define differing methods of treatment, storage, and disposal for contaminated material. Contaminated materials that may be landfilled can be categorized as follows:

- Contaminated soil
- Soil and debris treated by caustic washing
- Soil treated by thermal desorption
- Structural debris

The contaminated soil consists of excavated soil that is untreated. However, it is estimated that only a small portion of the soil will contain listed or characteristic hazardous waste. Metallic debris and soil from UXO clearance operations are included in the contaminated soil category. The second category includes soil and structural debris that has been treated by caustic washing to address agent contamination. The third category consists of soil treated by thermal desorption to remove organic contamination. The final category includes untreated debris from the demolition of structures.

5.1.2 Projected Waste and Landfill Volumes

Table 5.1 summarizes the estimated volume of material to be landfilled for five different remedial alternatives presented in the DAA. It is estimated that between 440,000 cubic yards (CY) and 4,400,000 CY of waste materials will require landfilling depending on the selected remedial action alternative. The preferred sitewide alternative in the DAA, Landfill/Caps Scenario, requires a waste volume of 2,300,000 CY.

For the purposes of this report, the minimum and maximum waste volumes shown in Table 5.1 were increased to 1,000,000 CY and 6,000,000 CY to account for future adjustments in waste volumes. Based on this projected minimum and maximum waste volume, three conceptual landfill models were developed to account for different waste volumes and to provide a total landfill capacity that accounts for the operational and intermittent cover materials that must be placed over the waste. Conceptual Model 2 is the preferred sitewide alternative and meets the waste volume requirements of 2,300,000 CY. The three models are as follows:

<u>Conceptual Model No</u>	<u>Total Landfill Volume (CY)</u>	<u>Waste Volume (CY)</u>
1	1,200,000	1,000,000
2	2,760,000	2,300,000
3	7,200,000	6,000,000

For conceptual design purposes, the total landfill volumes presented above include a 20 percent volume increase over the needed waste volume to account for operational and intermittent waste cover material.

5.1.3 Waste Generation Rates and Schedule

Waste generation rates at RMA will primarily be dependant on which of the five remedial action alternatives is selected and the duration of the remedial action. For this study, generation rates were estimated based on a schedule of 260 landfill operating days per year (5 operating days per week for 52 weeks). Additionally, two generation rates are estimated for each alternative in the DAA based on the development of remediation time frames with and without funding limitations. The funding

limitation case assumes an annual funding limitation of \$100 million for overall remediation activities, which lengthens the remediation period and decreases the generation rates. The remediation time frame includes landfill design, construction, and final closure. Table 5.2 presents the estimated waste generation rates for five different remedial alternatives presented in the DAA.

Waste generation rates have been estimated to be in the range of 98,000 CY to 1,100,000 CY of material per year without considering a funding limit. Assuming the \$100 million annual funding limit, waste generation rates are estimated to be in the range of 37,000 CY to 280,000 CY of material per year.

5.2 Regulatory Criteria

Section 4.2 of this report describes the regulatory criteria that apply to siting a landfill in Colorado. A detailed review of all possible current requirements, such as a review of applicable or relevant and appropriate requirements (ARARs) under CERCLA and the Superfund Amendments and Reauthorization Act of 1986 (SARA) was not conducted as part of the current scope of work. The criteria listed in Section 4.2 will likely form the basis for siting a facility under a variety of regulatory scenarios, such as a permitted facility under RCRA, a corrective action management unit (CAMU) under RCRA, or as an interim response action (IRA) under CERCLA. The specific regulatory criteria can be more fully developed once the regulatory framework for siting the facility is better defined.

5.3 Site-specific Considerations and Limitations

Following development of the range of landfill volume (1,000,000 CY, 2,300,000 CY, 6,000,000 CY), the landfill conceptual models were further developed by reviewing and evaluating site conditions and characteristics that could impact the construction of a hazardous waste landfill. Existing site data was reviewed to identify specific limitations and considerations that may need to be addressed

in

- The development of design alternatives
- The specific placement of the landfill within the preferred area

- Final design

5.3.1 Climate, Topography, and Surface Hydrology

The site's climate is semi-arid with an average annual precipitation of 15 inches. The majority of the yearly precipitation typically occurs between the months of March and August. The agricultural growing season is defined as the period between the last frost and the first frost, which averages about 150 days, however, soil temperatures are high enough to sustain plant growth for about 250 days of most years. Therefore, selection of the vegetative cover for the final cover system at final design should take this into consideration to establish the necessary erosion protection year round.

The ground surface in the vicinity of the preferred siting area generally ranges in elevation from about 5,280 feet above MSL to 5,225 feet above MSL and slopes towards the northwest. Figure 4.4 illustrates the onpost topography at RMA. In general, the area is a treeless plain. As illustrated in the 100-year floodplain map of RMA (Figure 4.2), there are no major drainage channels across the preferred landfill site. First Creek is a well-defined channel crossing the RMA to the east of the study area.

The footprints for the three conceptual models should be placed such that run-on to the landfill will be minimized and runoff can be effectively managed. As described in Sections 4.2 and illustrated in Figure 4.2, the site is not within the 100-year floodplain. Drainage can be designed to comply with regulations without any significant problems for any of the three proposed landfill volumes. There is sufficient area for ditches and other drainage facilities, existing slopes are acceptable or can be amended without excessive amounts of cut and fill, and there is a receiving ditch north of the footprint areas.

Run-on to the landfill can be prevented by constructing ditches and/or berms. A landfill perimeter berm could serve this purpose.

A more detailed description of the topography, and surface hydrology is included in Section 4.2.1 of this report.

5.3.2 Soil and Bedrock

The preferred landfill area is underlain by unconsolidated Quaternary alluvium and the Denver Formation. The alluvial soils encountered in the preferred landfill siting area generally consist of the following two types of material: (1) clay and sandy clay and (2) sand, silty sand and clayey sand with occasional gravel.

The depth to weathered bedrock (Denver Formation) in general follows the surface topography in the study area and ranges from approximately 5 feet to 60 feet. The areas where depth to bedrock is shallowest correspond with areas of high topographic elevation. The Denver Formation generally consists of three strata: claystone with interbedded siltstone, lignite, and sandstone; sandstone, and lignite/lignitic claystone. The alluvium is generally underlain by claystone; however, there are areas in the vicinity of the landfill siting area where Denver Formation channel sand units (sandstone) are in contact with the alluvium. Geologic cross sections are presented in Figures 4.21 through 4.28. A more detailed description of the geological and geotechnical study is included in Section 4.4.2.1 of this report.

Bedrock channel sands subcrop into the alluvium in three locations in the preferred landfill area, as was noted in Section 4.0 (see Figures 4.29, 4.30, and 4.31). Although there are no regulatory siting criteria that require avoiding sand units, siting the landfill such that the base of the landfill were placed above or into the subcrop sands could be a preferential pathway for leachate migration away from the facility. If possible, the specific siting should place the base of the landfill into claystone. If it is not possible to avoid the sand subcrop areas, the following alternatives are available:

- Apply appropriate landfill design technology to minimize the potential impacts associated with these features.

- Include wet/dry sand subcrop monitoring points in the facility's monitoring program because the saturated sand units could provide preferential migration pathways

5.3.3 Groundwater

As described in Section 4.2.3, there are two groundwater flow systems (the unconfined flow system and the confined flow system) in the preferred landfill area. The unconfined flow system is the primary flow system of concern because it is the first groundwater system encountered beneath the site. The unconfined flow system occurs at depths ranging from 20 to 70 feet below ground surface (bgs). The groundwater flow direction in the unconfined flow system is generally to the northwest. A groundwater surface contour map for the preferred area is presented in Figure 4.32. Figure 4.9 presents a contour map showing the depth to groundwater over the preferred area.

Hydrogeologic siting considerations at the preferred landfill site include maximizing the depth to groundwater and evaluating groundwater flow conditions with regard to the long-term groundwater monitoring. At the preferred landfill location (western half of Section 25), the depth to groundwater is greatest in the center of the area (Figure 4.32). For long-term groundwater monitoring of the landfill, it is preferable to place the landfill away from areas where groundwater mounds occur because groundwater flows radially from these areas and monitoring for potential leakage is difficult. A groundwater mound exists that straddles the boundary between Sections 25 and 36 (Figure 4.32). For these two reasons, siting the landfill within the central portion of western Section 25 is preferred.

5.3.4 Geologic Hazards

The location of the preferred landfill siting area was selected where no geologic hazards such as active faults, unstable areas, or poor foundation conditions are known to exist. Additional study may be required as a component of subsequent engineering design.

5.3.5 Environmentally Sensitive Areas

The location of the preferred landfill siting area was selected to avoid wetlands, floodplains, sites of historical significance, sensitive wildlife habitats, or other environmental sensitive areas.

5.3.6 Slope Stability Considerations

Proposed preliminary excavation plans should be developed such that the landfill bottom will be located in or near a competent bedrock formation (Denver Formation) consisting of either shale, sandstone, lignite, or claystone. Slope failure within these formations is unlikely, considering the slope geometry and proposed final elevations.

5.3.7 Settlement Considerations

The field and laboratory data indicate the alluvial soils are generally medium dense/stiff to very dense/stiff. Assuming landfill geometry consisting of a 30-foot excavation and a 35-foot fill above grade, the net maximum additional surcharge on the subsurface soils will be on the order of 600 pounds per square foot (psf). This magnitude of surcharge may result in settlements on the order of 1/2 inch. This estimate should be checked during final design.

5.4 Conceptual Design Alternatives

The conceptual design parameters of a hazardous waste landfill are intended to

- Provide waste containment by separating the waste from the environment
- Prevent contaminant migration by encapsulating the waste
- Confirm facility performance by planning, scheduling, and implementing a periodic site monitoring program

The primary features of a hazardous waste landfill containment system are a liner system and cover (cap) system that completely enclose the waste. Containment system components and alternatives are discussed in detail in sections below and are as follows:

- Liner Systems
- Leachate Collection and Removal Systems
- Gas Management Systems
- Final Cover Systems
- Performance and Environmental Monitoring System

5.4.1 Liner Systems

Liner systems include multiple layers consisting of a combination of geomembranes/flexible membrane liners (FML), compacted clay liners (CL) or geosynthetic clay liners (GCL), and granular soil or geosynthetic drainage layers. The liner system is designed to minimize the release of hazardous waste or hazardous waste constituents to the environment.

Six liner systems were evaluated using EPA's Hydrologic Evaluation of Landfill Performance (HELP) Model, as shown below:

Landfill Liner System Alternatives

Layer (Top to Bottom)	Liner System No. 1	Liner System No. 2	Liner System No. 3	Liner System No. 4	Liner System No. 5	Liner System No. 6
1 Geomembrane	60-mil HDPE	60-mil HDPE	60-mil HDPE	60-mil HDPE	60-mil HDPE	60-mil HDPE
2 Barrier	GCL	CL	GCL	GCL	CL	CL
3 Drainage	Geonet	Geonet	Geonet	Geonet	Geonet	Geonet
4 Geomembrane	60-mil HDPE	60-mil HDPE	60-mil HDPE	60-mil HDPE	60-mil HDPE	60-mil HDPE
5 Barrier	GCL	GCL	CL	CL	CL	CL
6 Geomembrane	N.A.	N.A.	N.A.	40-mil HDPE	N.A.	40-mil HDPE

HDPE High-density polyethylene
 CL 3 feet of compacted clay
 GCL Geosynthetic clay liner
 N.A Not applicable

Cross sections of each liner system are presented in Figure 5.1. The purpose of the tertiary geomembrane included with Liner System Nos. 4 and 6 is to provide a moisture barrier between the moisture-conditioned compacted clay liner and the drier in situ soils. The results of the evaluation are presented in detail in Section 5.5.1.1.